

November 19, 2018

Amanda Lessard, Town Planner Town of Windham 8 School Road Windham, ME 04062

Re: Resubmission – Preliminary Subdivision Review

Gray Road Retirement Community

Weld, LLC - Applicant

Dear Amanda:

On behalf of Weld, LLC we have prepared the enclosed plans and supporting material reflecting the revisions to the Gray Road Retirement Community at the intersection of Gray Road (Route 202) and Swett Road. We have made several plan revisions to mitigate concerns raised about the placement of septic systems and the preservation of buffer areas adjacent to the wetlands.

The number of proposed dwelling units has been increased from 12 to 14, and we have "flipped" the alignment of the roadway and units on the back half of the project so that the buildings are located adjacent to the wetlands. This was done to accommodate the neighbors' concerns about roadway snow being plowed into the wetland area and the elimination of all buffers to the wetland. The revised plans show a forested buffer maintained along the wetland area and we have eliminated the previously proposed wetland impact. The area that will be graded for the downslope of the wastewater disposal field will be allowed to revert back to natural conditions after development to further increase the buffer length over time.

The previous plans included one small leach field providing wastewater disposal for two units and one larger leach field that accommodated wastewater disposal for 10 units. The revised plan proposes four leach fields to better distribute the wastewater over the project's land area. Additionally, two of the septic systems have been designed with Advanced Treatment to reduce the concentration of Nitrates prior to entering the disposal fields. A Nitrate-Nitrogen Assessment Report prepared by Summit Geoengineering Services is enclosed which provides more detailed information on the design of the wastewater disposal systems and advanced treatment units.

Upon your review of this information, please let us know if you have any questions or require any additional information.

Sincerely,

DM ROMA CONSULTING ENGINEERS

Dustin M. Roma, P.E.

Dustin Roma

President



November 16, 2018

Summit #18323

Dustin Roma, P.E.

DM Roma Consulting Engineers
PO Box 1116
Windham, ME 04062

Reference: Nitrate-Nitrogen Assessment

Proposed Gray Road Retirement Community - Gray Road, Windham, Maine

Dear Dustin:

Summit Geoengineering Services (SGS) performed this nitrate-nitrogen assessment to estimate the groundwater quality impact caused by the proposed subsurface wastewater disposal systems for the Gray Road Retirement Community. The proposed development consists of fourteen two-bedroom retirement home units on approximately 11.4-acres of forestland located on the west side of Gray Road in Windham, Maine. A site location map showing the site and vicinity is provided as Attachment 1.

Information used for our evaluation includes a development plan provided by DM Roma Consulting Engineers (DM Roma), subsurface wastewater disposal system applications (HHE-200s) prepared by SGS, and published soil maps, geologic maps and literature.

Site Setting

The site is located on a north-south trend ridge on the west side of Gray Road (Route 202) as shown on Figure 1 in Attachment 1. A surface water drainage divide is located in the approximate center of the site. The eastern portion of the site drains easterly toward Gray Road. The western portion of the site drains westerly to a large wetland area located partially on the property, and partially on adjoining properties on Swett Road.

Review of Maine Geological Survey maps¹ indicate the surficial geology at the site and vicinity is mapped as glacial till with areas of exposed or shallow bedrock. No mapped significant sand and gravel aquifers are located within approximately 1 mile of the property. Glacial till consist of moderate compact, poorly sorted, weakly to non-stratified mixture of silt, sand, pebble, cobbles and boulders deposited by glacial ice, and is generally deposited directly on top of bedrock.

SGS observed soils on-site during field explorations for siting the proposed subsurface wastewater disposal fields. Soils at the proposed disposal field locations consist predominately of sandy loam to fine sandy loam with trace to some cobbles and gravel over bedrock. Sandy loam overlying silt loam soils (glaciolacustrine deposits) were observed in the low-lying areas near the wetlands on the western portion of the site. Depths to bedrock at the proposed disposal field locations range from approximately 1 foot to more than 3 feet below the ground surface. Bedrock outcrops are visible in some area of the property.

¹ https://www.maine.gov/dacf/mgs/pubs/index.shtml



Water Supply

The proposed development will be served by public water via a Portland Water District water main on Gray Road (Route 202). Properties on Gray Road in the site vicinity are served by public water. Adjoining properties to the south and west on Swett Road are served by on-site wells.

Wells located within 100 feet of the proposed disposal fields were field located by SGS using tape measurements to nearby property boundary markers and are shown on plans prepared by DM Roma. The proposed disposal fields are located more than 100 feet from existing water supply wells.

Subsurface Wastewater Disposal Fields

The proposed development will be served by four subsurface wastewater disposal systems, as shown on the HHE-200 applications provided as Attachment 1. Design information for the proposed subsurface wastewater disposal systems are summarized in the table below.

Subsurface Wastewater Disposal Systems

System Designation	Units Served	Design Flow (GPD)	Subsurface Wastewater Disposal System Description
А	1 thru 4	720	Fuji Clean CEN10 Advanced Treatment System ² Plastic chamber disposal field
В	5 and 6	360	Eljen GSF disposal field
С	7 thru 10	720	Fuji Clean CE10 Advance Treatment System ³ Stone bed disposal field
D	11 thru 14	720	Eljen GSF disposal field

Notes:

- 1. GPD = gallons per day; mg-N/L = milligrams nitrogen per liter
- 2. Fuji Clean CEN10 treats septic tank effluent to 10 mg-N/L total nitrogen
- 3. Fuji Clean CE10 treats septic tank effluent to 20 mg-N/L total nitrogen

Nitrate-Nitrogen Assessment

A nitrate-nitrogen assessment was performed to estimate the distance from the disposal fields at which the concentration in groundwater would reach the Federal National Primary Drinking Water Standard and the Maine Maximum Exposure Guideline of 10 milligrams nitrogen per liter (mg-N/L). The average concentration of nitrate in septic tank effluent discharged from the disposal field used in this assessment is 40 mg-N/L.²

The concentration of nitrate-nitrogen in groundwater downgradient of the disposal fields will reduce as it flows away from the disposal field and mixes with groundwater, is removed by vegetation, or is converted to nitrogen gas by soil microbes in wetland areas (denitrification).

² MEDEP, Site Location of Development Permit Application (October 2015) Section 17.B.2.(a).



Based on our understanding of site geology, treated septic tank effluent will drain to the disposal field and infiltrate downward through unsaturated soil until a seasonally perched water table above the bedrock surface is encountered. Thereupon flow is lateral and hydraulically downgradient. The direction of shallow groundwater flow for the subsurface conditions observed at this site is downhill (topographically downgradient).

The distance at which groundwater downgradient of disposal fields reaches 10 mg-N/L (plume length) was estimated using a three-dimensional analytical solution^{3,4} for a point source in a uniform flow field. Variables used for the calculations include the permeability and effective porosity of soils, hydraulic gradient, and the daily mass of nitrate-nitrogen applied to groundwater. The analytical solution does not consider nitrogen removal by soil microbes, vegetation or sorption. Input parameters for the analytical point source solution are summarized in the table below.

Analytical Solution Input Parameters

Parameter	Value	Source Reference
Permeability	1.2 feet/day	Estimated based on the range of permeabilities for Paxton fine sandy loam (1.2 to 4.0 ft/day) listed in the Cumberland County Soil Survey 5
Effective Porosity	0.18	Published average value for silt ⁶
Hydraulic Gradient	Varies	Half of the average topographic gradient upgradient/downgradient of the disposal field.

The treatment capacity of wetlands⁷ to remove nitrogen from shallow groundwater through plant uptake and microbial activity is significant. In instances where the nitrate plume for a disposal field intersects a mapped wetland area on the property prior the reaching 10 mg-N/L, the wetland's capacity to provide nitrate removal was evaluated. Research into the capacity of planted and unplanted wetlands to remove nitrogen at concentration similar to those in treated septic system effluent show nitrogen removal rates in excess of 95% for planted wetlands and removal rates of 25% to 36% in unplanted (natural) wetlands. Using the 25% nitrate removal rate in Lin et. al. (2002) ⁸ a wetland area nitrate removal rate of 0.0825 grams per square meter is calculated.

Results of our nitrate-nitrogen assessment are summarized in the table below. The table includes the methodology of evaluation used for each of the four proposed subsurface wastewater disposal systems and the estimated 10 mg-N/L plume length.

³ Baetsle, L.H. (1969), Migration of Radionuclides in Porous Media; Progress in Nuclear Energy, Series SIL, Health Physics. Pergamon Press, pp. 707-730.

⁴ Chang, et al. (1998). Utilizing Baetsle's Equation to Model the Fate and Transport of MTBE in Groundwater, Proceedings of the Petroleum Hydrocarbons and Organic Chemicals in Ground Water Prevention, Detection, and Remediation Conference, Houston, TX.

⁵ USDA Soil Conservation Services (1974), Soil Survey of Cumberland County, Maine.

⁶ Fetter, C.W. (1994). Applied Hydrogeology, 3rd Edition, Prentice Hall

⁷ US EPA (2005), Riparian Buffer Width, Vegetative Cover, and Nitrogen Removal Effectiveness: A review of Current Science and Regulations. EPA/600/R-05/118.

⁸ Lin, et. al. (2002), Effects of macrophytes and external carbon sources on nitrate removal from groundwater in constructed wetlands. Environmental Pollution, v. 119, pp. 413-420.



Nitrate-Nitrogen Assessment Summary

System Designation	10 mg-N/L Plume Length	Methodology of Evaluation
А	0 Feet	Total nitrogen concentration in treated effluent from the Fuji Clean CEN10 unit is 10 mg-N/L.
В	200 feet extending easterly towards Gray Road	Three-dimensional analytical point source solution using input parameters noted above.
С	Terminates in on-site	Nitrate-nitrogen in groundwater downgradient of the subsurface wastewater disposal fields will flow into the wetland area downgradient
D	wetlands	of the disposal fields, and be treated to less than 10 mg-N/L. Refer to calculations provided in Attachment 3.

Conclusion:

Proposed subsurface wastewater disposal systems B, C and D will not result in an increase of nitratenitrogen above 10 mg/L in groundwater at the property boundary.

Results of our analysis indicate the proposed subsurface wastewater disposal system A will result in an increase of nitrate-nitrogen above 10 mg/L in groundwater at the property boundary along Gray Road. Given that public water is available to future developments in the area of the plume from disposal field A and there are no known water supply wells near the plume, we proposed that the applicant request a waiver to the Town of Windham Land Use Ordinance for meeting the nitrate-nitrogen drinking water standard requirement this disposal field.

Our findings are based on our interpretation of site conditions and the information provided to us. If there are changes in lot layout, proposed septic system design flows, or significant changes in disposal field size, we request the opportunity to review the changes and conduct further analysis as necessary to confirm the changes do not alter our conclusions.

Sincerely yours,

Summit Geoengineering Services

Stephen B. Marcotte, C.G., L.S.E.

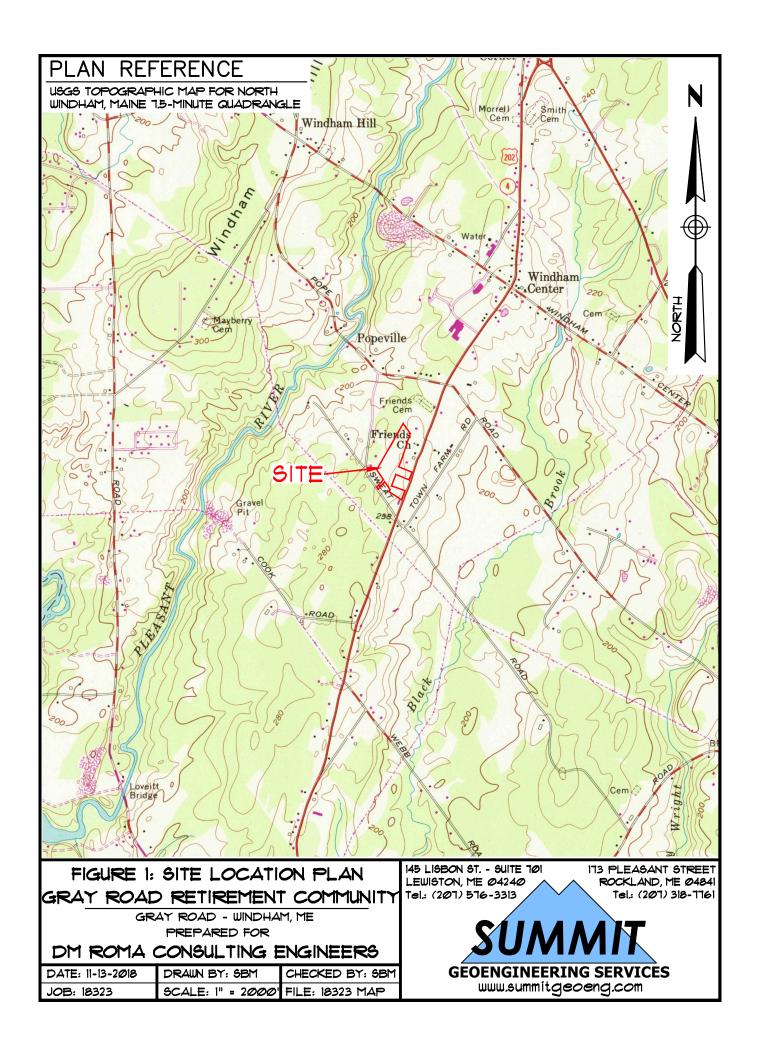
Senior Geologist

Enclosures





Attachment 1Site Location Map

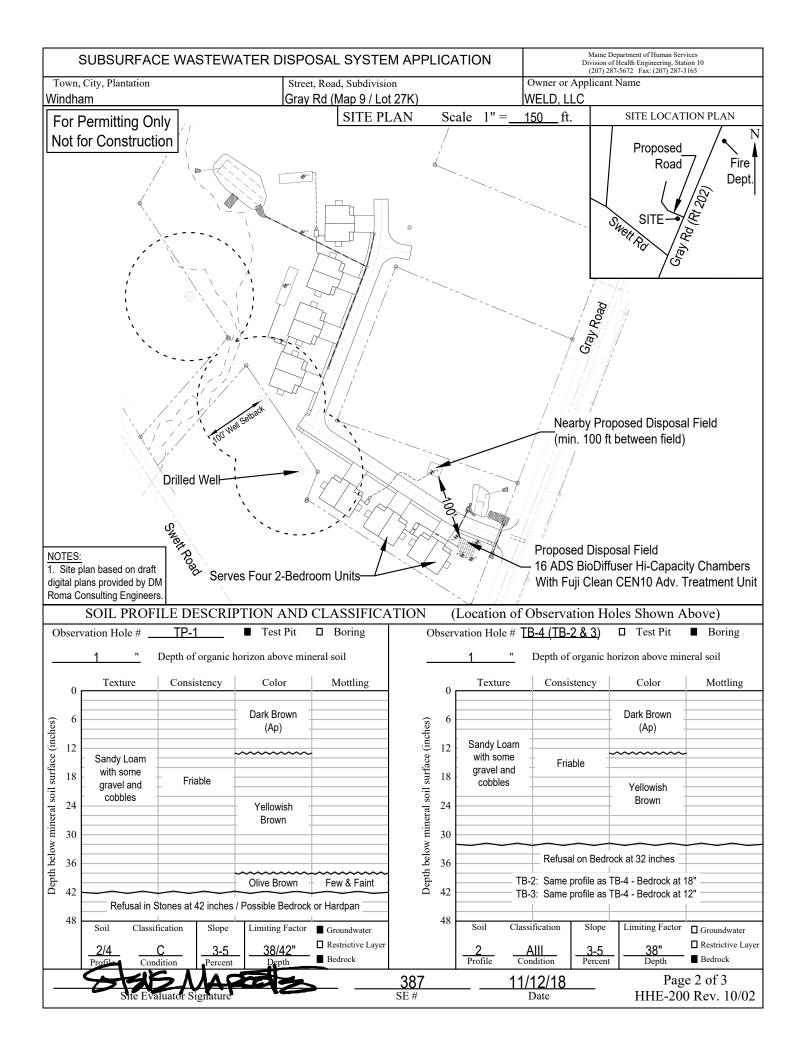


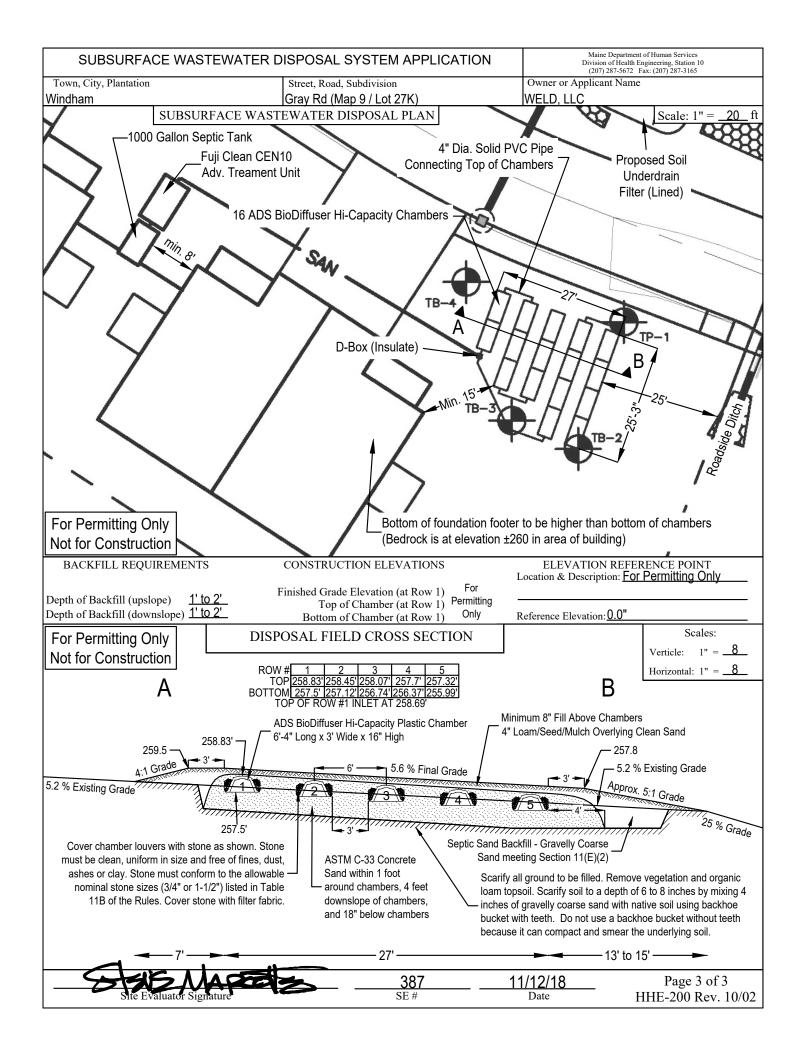


Attachment 2

Subsurface Wastewater Disposal System Applications (HHE-200s)

		ASTEWATER DISP	OSA	AL SYSTEM	ΛAF	PPLICA	TION	Maine Department of Human Services Division of Health Engineering, 10 SHS (207) 287-5672 Fax: (207) 287-3165		
/////////PF	RÓPÉRTÝ	LOCATION ////////////////////////////////////		>> C	AUTI	ON: LPI AF	PPROVAL F	REQUIRED <<		
City, Town, or Plantation	Vindham		Town	/City			'''' 0	Permit#		
Street or Road G	Gray Rd (I	Map 9 / Lot 27K)	Date	Permit Issued/	/_	1	nitting Onl Construction	I Double Fee Charged LI		
Subdivision, Lot #	ray Poad	Potiroment Community						L.P.I. #		
		Retirement Community		Local Plumbing Inspector Signature						
Name (last, first, MI)		☐ Owner ☐ Town ☐ The Subsurface Wastewater Disposal System shall not be installed until a								
WELD, LLC		Permit is issued by the Local Plumbing Inspector. This Permit shall								
Mailing Address of P(O Box 13	361 l			•		• .	al system in accordance		
O		ME 04062		with this application and the Maine Subsurface Wastewater Disposal Rules.						
	<u>viriuriarri,</u>	IVIL 04002								
Daytime Tel. #						al Tax Map#_		<u></u>		
I state and acknowledge th	hat the informat stand that any fa	ANT STATEMENT ion submitted is correct to the best of alsification is reason for the Department a Permit.			the inst		ed above and fo	und it to be in compliance tion. (1st) date approved		
Signature	re of Owner or A	'''''''''''''''''''''''''''''''''''''		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		g Inspector Sign	ature	(2nd) date approved		
<u> </u>		THIS APPLICATION	,,,,	INFORMATION	<u>///</u>	<u>////////</u>	///////			
TYPE OF APPLICA ■ 1. First Time System	-		REQUI	RES				COMPONENTS gineered System		
2. Replacement Sys		■ 1. No Rule Variance□ 2. First Time System Variance	_					graywater & alt. toilet)		
Type replaced:		a. Local Plumbing Inspecto	r Appro					, specify:		
Year installed:		☐ b. State & Local Plumbing I		or			engineered ir ing Tank,	reatment Tank (only) gallons		
☐ 3. Expanded System	m	□ 3. Replacement System Varia □ a. Local Plumbing Inspecto				☐ 6. Non-	engineered Di	isposal Field (only)		
a. <25% Expansion		☐ b. State & Local Plumbing I					arated Laundry	y System red System (2000 gpd or more)		
☐ b. >= 25% Expans ☐ 4. Experimental Sys		☐ 4. Minimum Lot Size Variance)					nent Tank (only)		
☐ 5. Seasonal Convers		☐ 5. Seasonal Conversion Perm	nit					sal Field (only)		
SIZE OF PROPER	RTY	DISPOSAL SYSTEM TO SE	RVE		■ 11. Pre-treatment, specify: Fuji Clean CEN10 □ 12. Miscellaneous Components					
	□SQ. FT.	☐ 1. Single Family Dwelling Unit, No.						•		
	ACRES	■ 2. Multiple Family Dwelling, No. o	of Units: (<u>4) 2-</u> Bedroom			TYPE OF WATER SUPPLY 1. Drilled Well 2. Dug Well 3. Private				
SHORELAND ZON	NING	3. Other:(specify)								
☐ Yes I	■No	Current Use Seasonal Year F	Round	Undeveloped		4. Public	5. Other			
		////DESIGN DETAILS (SÝSŤÍ	ÉM LAYOUT SH	ÍÓWÍ	Ń ÓN PÁGE	<u> </u>			
TREATMENT TA		DISPOSAL FIELD TYPE & S		GARBAGE DI	SPOS	AL UNIT		DESIGN FLOW		
	ıji CEN10	1. Stone Bed 2. Stone Trend	h	■ 1. No □ 2. Ye		-	720	gallons per day		
a. ivegulai	uire septic	■ 3. Proprietary Device□ a. cluster array□ c. Linear		If Yes or Maybe, s ☐ a. multi-compart				ASED ON:		
Π ₂ Plastic tan	nk - 1,000	■ b. regular load □ d. H-20 loa	d	b tanks in s		Ialik		IA (dwelling unit(s)) IC (other facilities)		
3. Other:	. proposed	☐ 4. Other:		c. increase in ta		acity		ean CEN10 approved for 75%		
CAPACITY:	00_ gal	SIZE: 800 sf stone bed equivale	ent _	d. Filter on Tank			I	disposal field area - see attached		
SOIL DATA		DISPOSAL FIELD SIZING		EFFLUENT/EJ		R PUMP		7% reduction proposed		
	AIII	☐ 1. Medium2.6 sq. ft. / gpd		1. Not Required				NATER METER DATA		
at Observation Hole #_		2. MediumLarge 3.3 sq. f.t / g	pd	■ 2. May Be Requ	iired			TITUDE AND LONGITUDE		
Depth 12 "	100	☐ 3. Large4.1 sq. ft. / gpd		☐ 3. Required			LatN4:	at center of disposal area 3d46m39.66s		
of Most Limiting Soil Fa	actor	☐ 4. Extra Large5.0 sq. ft. / gpd		Specify only for er	nginee	red systems:	Lon. W7	<u>0 d 25 m 0.60</u> s		
Bedrock	,,,,,,,,	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	,,,,	DOSE:	 _	llons	if g.p.s. state	e margin of error: <u>20'</u>		
		/////////////////SITE EV/	ďĽŃŸ.	TOR STATEME	NT//	<u> </u>	<u> </u>	<u> </u>		
I certify that on	11/5/2018	(date) I completed a sit	e eval	uation on this pro	operty	y and state	that the dat	ta reported are accurate and		
that the proposed	system is i	n compliance with the State o	f Main	ne Subsurface W	astev/	vater Dispo	sal Rules (1	I0-144A CMR 241).		
S-1-	がらん	MODE		387		1	1/12/18			
Site	e Evaluator			SE#			Date			
Ston	nhan D N	/larcotto		(207) 020 (วลกก) omo	rcotto@c	cummitacoena com		
	<u>phen B. I</u> Evaluator I	Viai Colle Name Printed		(207) 939-2 Telephone Nui				summitgeoeng.com ail Address		
Designed with Ser		tamo i filitou		relephone Mul	IIIDEI		L1110	Page 1 of 3		
		ons from the design should be	e confi	irmed with the Si	te Ev	aluator.		HHE-200 Rev. 08/2011		







Paul R. LePage, Governor Tel. (207) 287-2070 Ricker Hamilton, Commissioner

Drinking Water Program

Department of Health and Human Services Maine Center for Disease Control and Prevention 286 Water Street 11 State House Station Augusta, Maine 04333-0011 Tel.: (207) 287-8016; Fax: (207) 287-9058 TTY Users: Dial 711 (Maine Relay)

Fax (207) 287-4172

April 9, 2018

Fuji Clean USA, LLC

Attn.: Scott Samuelson, Managing Director

41-2 Greenwood Road Brunswick, ME 04011

Subject: Disposal Field Size Reduction, Fuji Clean Models CEN5, CEN7, CEN10, and CEN21

Dear Mr. Samuelson:

The Division of Environmental and Community Health has reviewed your proposal for 75 percent reductions in disposal field sizing compared to the standard sizing requirements in the Maine Subsurface Wastewater Disposal Rules for systems which incorporate Fuji Clean Models CEN5, CEN7, CEN10, and CEN21 wastewater treatment systems. This request is predicated upon the ability of the Fuji Clean system to produce BOD5 and TSS levels below 10 mg/l, each as verified in the NSF report dated April 2015.

The Division approves the request for reduced disposal field area and reduced separation distances as follows:

- 1. A minimum separation distance of 12 inches shall be maintained between the seasonal high groundwater table and the lowest elevation of the system's disposal field;
- 2. A minimum separation distance of 12 inches shall be maintained between bedrock and the lowest elevation of the system's disposal field;
- 3. Stone beds and trenches are allowed a 75 percent reduction in size, based upon the standard sizing requirements of the Rules;
- 4. Proprietary devices such as but not limited to plastic chambers and gravel-less pipe trenches are allowed a 75 percent reduction in size based upon the standard sizing requirements of the Rules, absent prohibitions by manufacturers; and
- 5. Maintenance agreement contracts must be included with all system installations. Terms and duration of the contracts shall be in accordance with Fuji Clean's company policies.

This letter supersedes the letter dated October 12, 2016.

Page 2

Because installation and maintenance has a significant effect on the working order of onsite sewage disposal systems, including their components, the Division makes no representation or guarantee as to the efficiency and/or operation of this system.

Should you have any questions, please feel free to contact me at (207) 287-5695, or by fax at (207) 287-4172.

James A. Jacobsen

James A. Jacobsen, Environmental Specialist IV
Division of Environmental and Community Health

Drinking Water Program Engineering Review Team

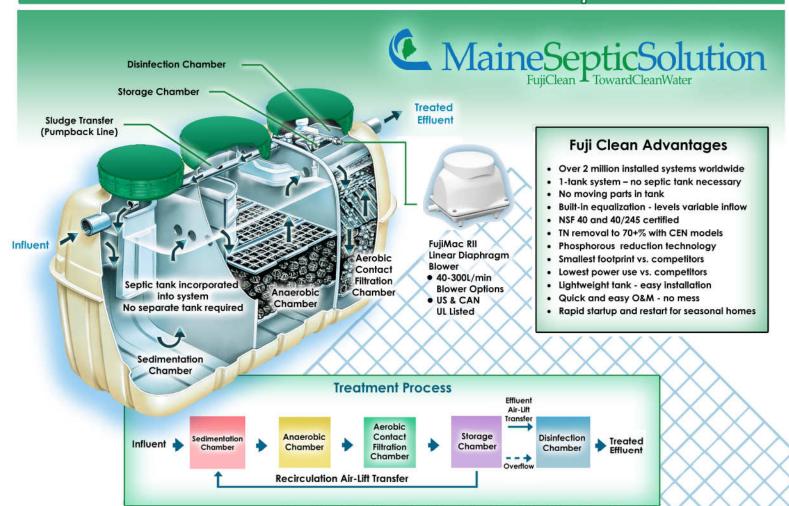
286 Water Street, Augusta, ME 04333

e-mail: james.jacobsen@maine.gov

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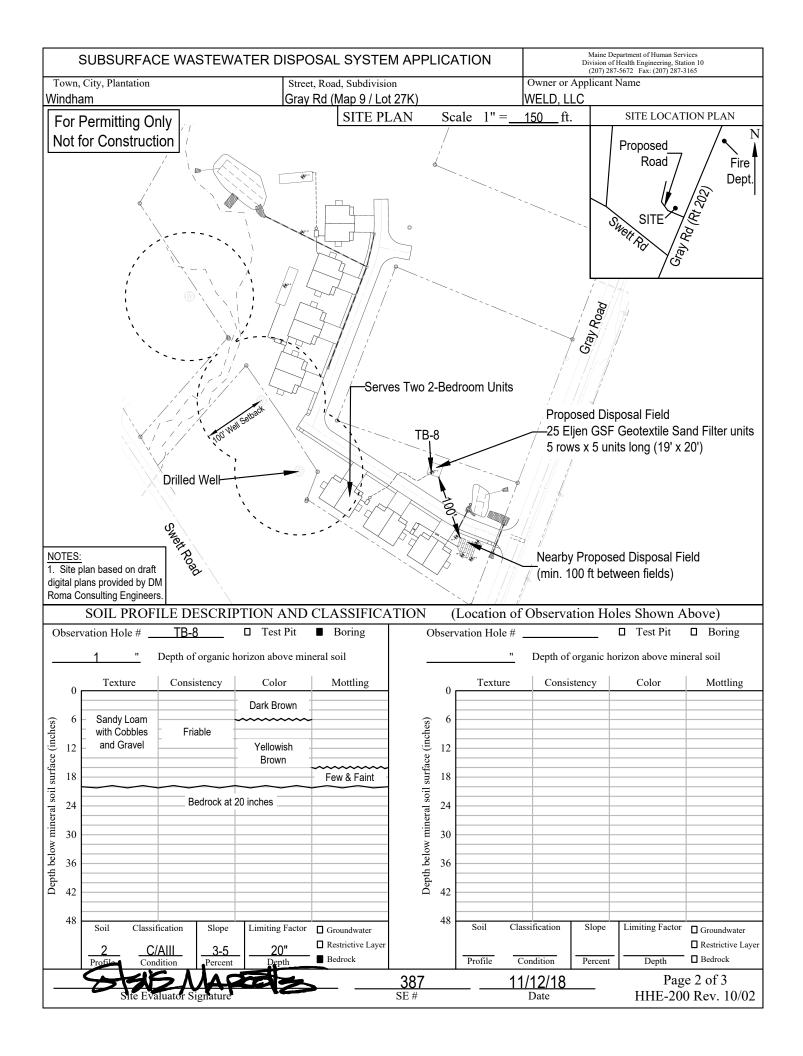
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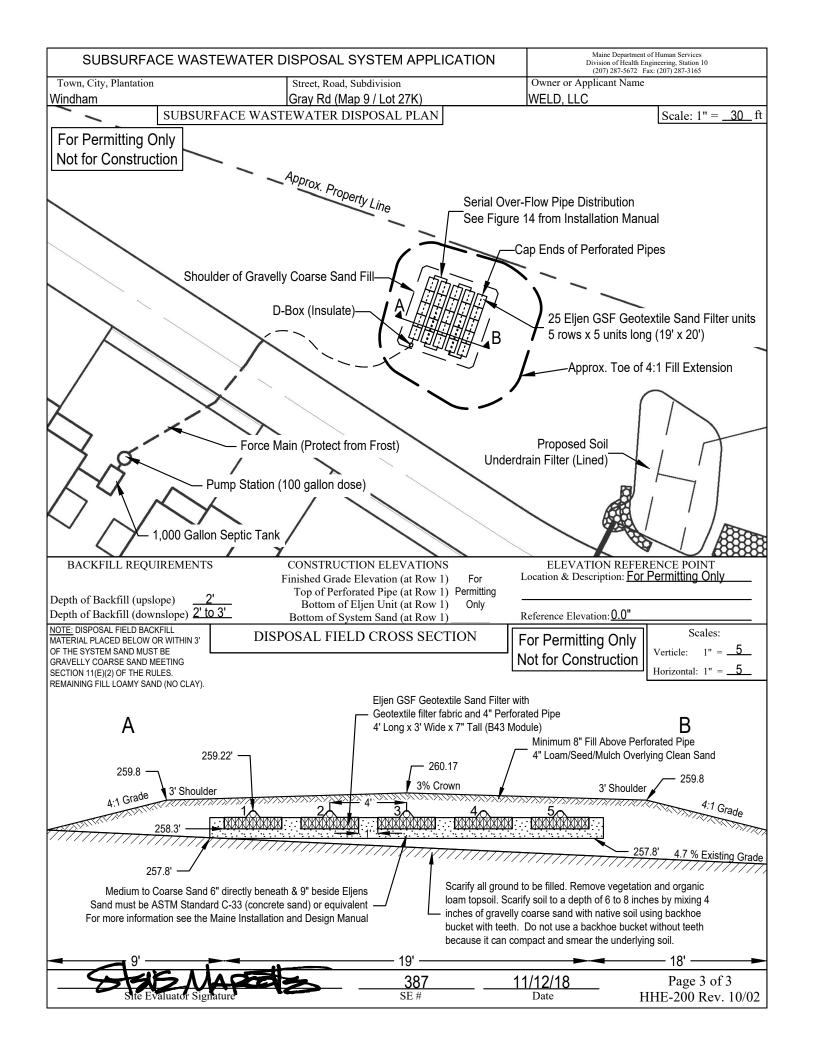
MODEL CE & CEN SERIES Technical Specification Sheet



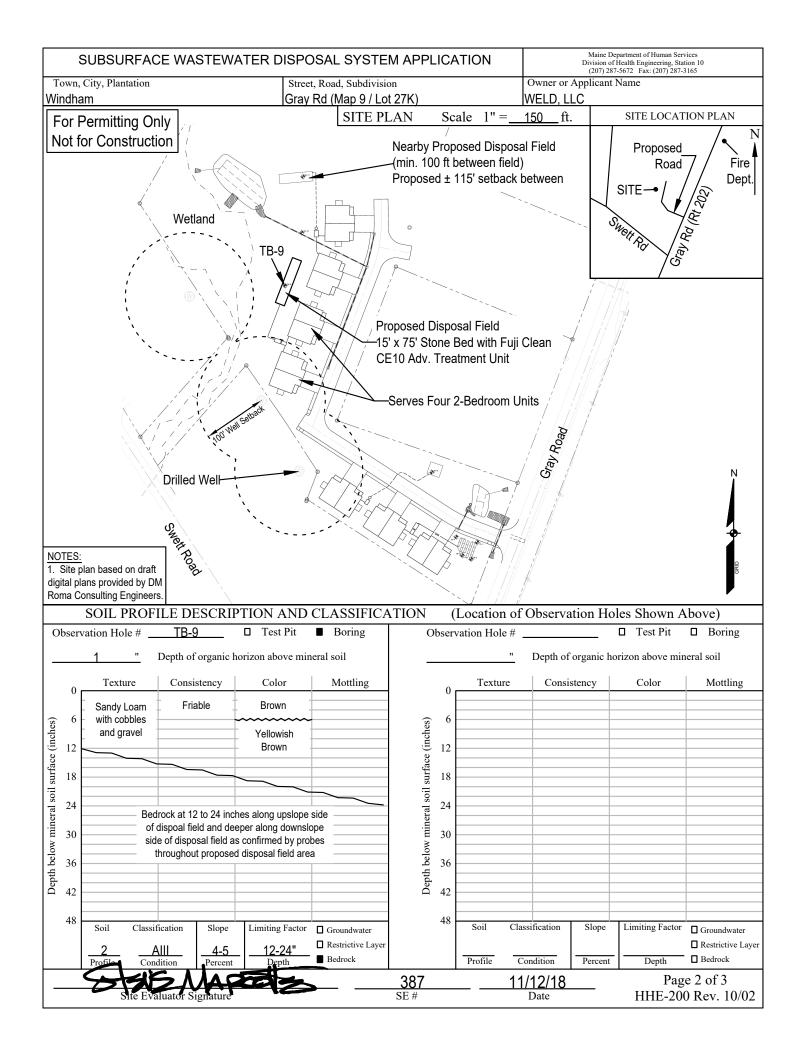
Design Specification Table	CE Series BOD, TSS, TN							CEN Series BOD, TSS, Enhanced TN				
MODEL:	CE5	CE7	CE10	CE14	CE21	CE30	CE6KG	CEN5	CEN7	CEN10	CEN21	
Load Hydraulic (GPD)	360	540	720	1,000	1,900	2,700	6,000	360	540	720	1,900	
EFFLUENT (assumes domestic stren	gth influent):											
BOD – Effluent (mg/L)	10-20	10-20	10-20	10-20	10-20	10-20	10-20	10	10	10	10	
BOD (removal pounds/day)	.52	.73	1.04	1.46	2.08	3.12	6.93	.69	.97	1.38	2.9	
TSS (mg/L)	10-20	10-20	10-20	10-20	10-20	10-20	10-20	10	10	10	10	
TN (mg/L)	10-20	10-20	10-20	10-20	10-20	10-20	10-20	10	10	10	10	
BLOWER DETAIL:												
Blower Model	MAC80R	MAC80R	MAC100R	MAC100R	MAC150R	MAC200R	MAC200R (3)	MAC80R	MAC100R	MAC100R	MAC200	
Normal Pressure (kPa)	15	15	18	18	20	20	20	15	18	18	20	
Airflow Volume (CFM; L/Min.)	2.8 CFM 80 L/MIN	2.8 CFM 80 L/MIN	3.5 CFM 100 L/MIN	3.5 CFM 100 L/MINJ	5.3 CFM 150 L/MIN	7.0 CFM 200 L/MIN	21.0 CFM 600 L/MIN	2.8 CFM 80 L/MIN	3.5 CFM 100 L/MIN	3.5 CFM 100 L/MIN	7.0 CFM 200 L/MI	
Power Use (kWh/day)	1.1	1.1	1.6	1.6	2.4	3.4	10.2	1.1	1.6	1.6	3.4	
Weight (lbs.)	11	11	11	11	13	13	13 x 3	11	11	11	13	
Outlet Diameter (OD, inches)	0.70	0.70	0.70	0.70	1.0	1.0	1.0 x 3	.070	0.70	0.70	1.0	
TANK DETAIL:												
Material			Fibre	-Reinforced I	Plastic				Fibre-Reinfo	rced Plastic		
Height (inches)	61.8	65.7	73.6	77.4	81.3	87.2	87.2	65.7	73.6	77.4	87.2	
Length (inches)	85	95.7	98.8	118.9	152.8	183.7	434.7	95.7	98.8	118.9	183.7	
Width (inches)	43.7	49.2	56.7	68.9	72.4	78.3	115.3	49.2	56.7	68.9	78.3	
Weight (lbs.)	397	463	705	926	1,168	1,543	2,900	463	705	926	1,543	
Inlet Invert (inches)	49	53	61	62	65	71	67	53	61	62	71	
Outlet Invert (inches)	47	51	59	59.5	63	69	64	51	59	59.5	69	
Access Ports Quantity & Diameter (inches)	3@20"	2@20" 1@24"	2@20" 1@24"	2@20" 1@24"	2@20" 1@24"	2@20" 1@24"	4@24"x24" 3@24"x48"	2@20" 1@24"	2@20" 1@24"	2@20" 1@24"	2@20" 1@24"	
Tank Volume Total (gallons)	545	749	1,069	1,498	2.252	3,199	7,267	749	1.069	1,498	3,199	

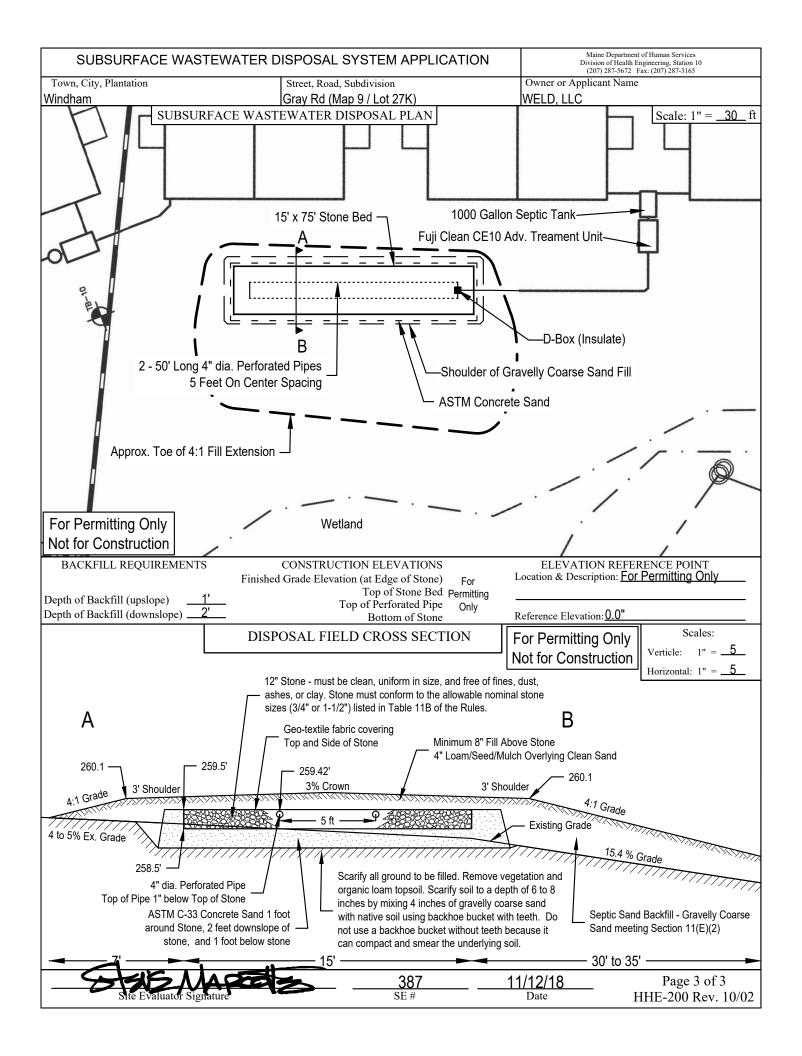
		ASTEWATER DISP	OSA	AL SYSTEM	ΛAF	PPLICA	TION	Maine Department of Human Services Division of Health Engineering, 10 SHS (207) 287-5672 Fax: (207) 287-3165		
//////P	RÓPÉRTY	LOCATION ////////////////////////////////////		>> C	AUTI	ON: LPI AI	PPROVAL F	REQUIRED <<		
City, Town, or Plantation	Vindham		Town	/City				Permit#		
Street or Road	Gray Rd (Map 9 / Lot 27K)	Date	Permit Issued/	/_	1	nitting Onl Construction	' I Double Fee Charged LI		
Subdivision, Lot #	Subdivision, Lot # Gray Road Retirement Community							L.P.I. #		
///// OWNER	DIAY KUAU	NT INFORMATION		Local Plui	mbing I	nspector Signat	ure			
Name (last, first, MI)	VAFFLICA	■ Owner			14/ /	. 5:		Owner Town State		
WELD, LLC	The Subsurface Wastewater Disposal System shall not be installed until a Permit is issued by the Local Plumbing Inspector. This Permit shall									
Mailing Address of P			•		•	al system in accordance				
O					•	stewater Disposal Rules.				
l V	<u>vinanam,</u>	ME 04062		· · · · · · · · · · · · · · · · · · ·						
Daytime Tel. #				M	unicipa	al Tax Map#_	Lot #	<u> </u>		
I state and acknowledge t	that the informates	ANT STATEMENT ion submitted is correct to the best of alsification is reason for the Department a Permit.			the inst		ed above and fo	und it to be in compliance tion. (1st) date approved		
Signatu	re of Owner or	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	,,,,	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		g Inspector Sigr	nature	(2nd) date approved		
<u> </u>				INFORMATION	///	<u>///////</u>	////////			
TYPE OF APPLIC	-	THIS APPLICATION	REQUI	RES				COMPONENTS ineered System		
■ 1. First Time System 2. Replacement Sy		■ 1. No Rule Variance	_					graywater & alt. toilet)		
Type replaced:		□ 2. First Time System Variance□ a. Local Plumbing Inspecto		val				specify:		
Year installed:		☐ b. State & Local Plumbing I					-	Freatment Tank (only)		
☐ 3. Expanded Syste	em	☐ 3. Replacement System Varia			☐ 5. Holding Tank, ☐ 6. Non-engineered Di					
☐ a. <25% Expans	ion	□ a. Local Plumbing Inspecto□ b. State & Local Plumbing I			☐ 7. Separated Laundry System ☐ 8. Complete Engineered System (2000 gpd or more) ☐ 9. Engineered Treatment Tank (only)					
□ b. >= 25% Expar		☐ 4. Minimum Lot Size Variance	•							
☐ 4. Experimental Sy☐ 5. Seasonal Conve		☐ 5. Seasonal Conversion Perm	nit			-		sal Field (only)		
SIZE OF PROPE		DISPOSAL SYSTEM TO SE	P\/F		☐ 11. Pre-treatment, specify: ☐ 12. Miscellaneous Components					
SIZE OF FROIL		☐ 1. Single Family Dwelling Unit, No		drooms:		∐ 12. Mis	cellaneous Co	mponents		
±11.4	□SQ. FT. ■ACRES	■ 2. Multiple Family Dwelling, No. o				TYPE	OF WATER S	SUPPLY		
SHORELAND ZO	NING	□ 3. Other:				☐ 1. Drilled Well ☐ 2. Dug Well ☐ 3. Private				
☐ Yes	■No	(specify) Current Use ☐ Seasonal ☐ Year F	Pound Lindayoloped			■ 4. Public □ 5. Other				
<i></i>		/////DESIGN DÉTAILS (S			ÍÓVÁÝ	ν οΝ ράςί	=3\////			
TREATMENT 1	<i>////////</i> TANK	DISPOSAL FIELD TYPE & SI		GARBAGE DI			9 <i>/////</i>	DESIGN FLOW		
■ 1. Concrete		☐ 1. Stone Bed ☐ 2. Stone Trend	h	■ 1. No □ 2. Ye	es 🗆 3	B. Maybe				
■ a. Regular		■ 3. Proprietary Device		If Yes or Maybe, s		-	360	gallons per day \SED ON:		
☐ b. Low Profile		a. cluster array c. Linear		a. multi-compar	tment t	tank		ASED ON: A (dwelling unit(s))		
2. Plastic		■ b. regular load □ d. H-20 loa	ıd	b tanks in s			☐ 2. Table 4	C (other facilities)		
☐ 3. Other:	000 _{GAL}	☐ 4. Other: SIZE: 1200 sf stone bed equivale	ent	☐ c. increase in ta☐ d. Filter on Tank		-		CALCULATIONS ther facilities—		
SOIL DATA		DISPOSAL FIELD SIZING		EFFLUENT/EJ						
	NDITION			☐ 1. Not Required			☐ 3. Section	4G (meter readings)		
2	C/AIII_	☐ 1. Medium2.6 sq. ft. / gpd		☐ 2. May Be Requ	ired			WATER METER DATA		
at Observation Hole #	<u>TB-8</u>	■ 2. MediumLarge 3.3 sq. f.t / g	pd	■ 3. Required				TITUDE AND LONGITUDE at center of disposal area		
Depth <u>20 "</u>		☐ 3. Large4.1 sq. ft. / gpd		Specify only for e	nainee	red systems	Lat <u>N4</u>	<u>3</u> d <u>46</u> m <u>40.77</u> s		
of Most Limiting Soil F Bedrock	of Most Limiting Soil Factor 4. Extra Large5.0 sq. ft. / gpd				-	llons	Lon. <u>W7</u>	0 d 25 m 0.60 s margin of error: 20'		
//////////////////////////////////////		//////////////////////////////////////	Δίίδ-	DOSE: 100 TÓR STÁTÉMÉI	 _		1111111			
1	44/5/004/			 			///////			
certify that on	11/5/2018			-		=		a reported are accurate and		
that the proposed	system is i	n compliance with the State o	t Main		astev	-	-	10-144A CMR 241).		
	72/	MARTIN		387		1	<u>1/12/18</u>	_		
Sit	te Evaluato	r Signatūre		SE#			Date			
Ste	phen B. I	Marcotte		(207) 939-2	2600) sma	arcotte@s	summitgeoeng.com_		
		Name Printed		Telephone Nu				ail Address		
Designed with Se	eptiCAD v5			•				Page 1 of 3		
Note: Changes to	o or deviati	ons from the design should be	e confi	irmed with the Si	te Ev	aluator.		HHE-200 Rev. 08/2011		





		ASTEWATER DISP	OSA	AL SYSTEM	ΙA	PPLICA	TION	Maine Department of Human Services Division of Health Engineering, 10 SHS (207) 287-5672 Fax: (207) 287-3165		
	PRÓPÉRTÝ	LOCATION /////////		>> C	AUTI	ON: LPI AF	PPROVAL F	REQUIRED <<		
City, Town, or Plantation	Windham		Town	/City		f_ 5	'''' 0 1	Permit#		
Street or Road (Gray Rd (Map 9 / Lot 27K)	Date	Permit Issued/	/_	II .	nitting Onl	- I Double Fee Charged LI		
Subdivision, Lot #	Gray Road	Retirement Community						L.P.I. #		
		NT INFORMATION//////		Local Plumbing Inspector Signature ☐ Owner ☐ Town ☐						
Name (last, first, MI)		The Subsurface	Waste	ewater Disposa	l System shall					
WELD, LLC		☐ Applicant		The Subsurface Wastewater Disposal System shall not be installed until a Permit is issued by the Local Plumbing Inspector. This Permit shall						
Mailing Address of	PO Box 13	361		authorize the ow	ner or	installer to ins	tall the dispos	al system in accordance		
Owner/Applicant	<i>M</i> indham.	ME 04062		with this application and the Maine Subsurface Wastewater Disposal Rules.						
Daytime Tel. #	,			M	unicipa	al Tax Map#_	Lot #	!		
I state and acknowledge	that the informaterstand that any f	ANT STATEMENT ion submitted is correct to the best of alsification is reason for the Department a Permit.			the inst		ed above and fo	und it to be in compliance ion. (1st) date approved		
Signat	ture of Owner or	Applicant Date		Local F	Plumbin	g Inspector Sign	ature	(2nd) date approved		
	////////			INFORMATION		////////		<u>/////////////////////////////////////</u>		
TYPE OF APPLIC		THIS APPLICATION	REQUIF	RES				COMPONENTS ineered System		
■ 1. First Time Syste		■ 1. No Rule Variance□ 2. First Time System Variance	•					graywater & alt. toilet)		
Type replaced:	•	a. Local Plumbing Inspecto	r Appro					specify:		
Year installed:		b. State & Local Plumbing I		or			engineered ir ing Tank,	reatment Tank (only) gallons		
☐ 3. Expanded Syste	em	☐ 3. Replacement System Varia ☐ a. Local Plumbing Inspecto					-	sposal Field (only)		
☐ a. <25% Expans ☐ b. >= 25% Expa		☐ b. State & Local Plumbing I					arated Laundry plete Engineer	red System (2000 gpd or more)		
4. Experimental S		4. Minimum Lot Size Variance				9. Eng	ineered Treatn	nent Tank (only)		
☐ 5. Seasonal Conv	rersion	☐ 5. Seasonal Conversion Perm	nit					sal Field (only) _{ecify:} <u>Fuji Clea</u> n CE10		
SIZE OF PROPI	ERTY	DISPOSAL SYSTEM TO SE			12. Miscellaneous Components					
±11.4	□SQ. FT.	□ 1. Single Family Dwelling Unit, No■ 2. Multiple Family Dwelling, No. o				TYPF	OF WATER S	SUPPLY		
±11. 7	■ACRES	3. Other:	i Omis.	(<u>4) 2-</u> Dearoom	☐ 1. Drilled Well ☐ 2. Dug Well ☐ 3. Private					
SHORELAND ZO		(specify)			■ 4. Public □ 5. Other					
Yes	■ No	Current Use Seasonal Year F			(A) (A (A	, , , , , , , , , , , , , , , , , , , 	,,,,,,,			
TREATMENT	TANK	DISPOSAL FIELD TYPE & S		GARBAGE DI			<u> </u>	DESIGN FLOW		
	ji CE10 does	■ 1. Stone Bed □ 2. Stone Trend		■ 1. No □ 2. Ye						
■ a. Regular r	not require	☐ 3. Proprietary Device		If Yes or Maybe, s		-	<u>720</u>			
L D. LOW FIGHIN	eptic tank - 1,000 gal.	a. cluster array c. Linear		a. multi-compar		tank		SED ON: A (dwelling unit(s))		
☐ 2. Plastic	proposed	☐ b. regular load ☐ d. H-20 loa☐ 4. Other:	ia	b tanks in s		ocity	2. Table 4	C (other facilities)		
□ 3. Other:	000 GAL	SIZE: 1230 sf including sidewall are	ea -	☐ c. increase in ta☐ d. Filter on Tank				CE10 approved for 50% reduction		
SOIL DAT	-A	DISPOSAL FIELD SIZING		EFFLUENT/EJ	ECTO	R PUMP		osal field area - see attached 8% reduction proposed		
_	ONDITION	☐ 1. Medium2.6 sq. ft. / gpd		☐ 1. Not Required			☐ 3. Section	4G (meter readings) WATER METER DATA		
2	AIII	2. MediumLarge 3.3 sq. f.t / g	bq	■ 2. May Be Requ	ired			TITUDE AND LONGITUDE		
at Observation Hole #	# <u>ID-9</u>	☐ 3. Large4.1 sq. ft. / gpd		☐ 3. Required			8	at center of disposal area		
of Most Limiting Soil	Factor	☐ 4. Extra Large5.0 sq. ft. / gpd		Specify only for e	nginee	red systems:	Lat. <u>N4.</u> Lon. <u>W7</u>			
Bedrocl	k		,,,,	DOSE:	 _	llons	if g.p.s. state	e margin of error:20'		
		/////////////////SITE EV/	ĄĻŲĄ.	TOR STATEME	NT//	<u> </u>	<u> </u>	<u>/////////////////////////////////////</u>		
I certify that on _	11/5/2018	(date) I completed a sit	e eval	uation on this pro	operty	y and state	that the dat	a reported are accurate and		
that the propose	d system is i	n compliance with the State o	f Main	ne Subsurface W	astev	vater Dispo	sal Rules (1	10-144A CMR 241).		
	3/2/	MARCO		387		1	<u>1/12/18</u>	<u> </u>		
S	ite Evaluato	r Signature		SE#			Date			
Ste	ephen B. I	Marcotte		(207) 939-2	2600) sma	rcotte@s	summitgeoeng.com_		
Site	e Evaluator	Name Printed		Telephone Nu				ail Address		
Designed with Solution Note: Changes		ons from the design should be	e confi	irmed with the Si	te Ev	aluator.		Page 1 of 3 HHE-200 Rev. 08/2011		







Tel. (207) 287-2070

Department of Health and Human Services Maine Center for Disease Control and Prevention 286 Water Street 11 State House Station Augusta, Maine 04333-0011 Tel. (207) 287-8016 Fax (207) 287-9058; TTY (800) 606-0215 Fax (207) 287-4172

October 12, 2016

Fuji Clean, LLC Attn.: Bennette D. Burkes, P.E. 1518 Willow Lawn Drive, Suite 300 Hnerico, VA 23230

Subject: Modified Approval for General Use, Fuji Clean System, CE and CEN Series

Dear Mr. Burkes:

The Division of Environmental and Community Health has reviewed your proposal for reductions in disposal field sizing and reduced separation form limiting factors for systems which incorporate the Fuji Clean System, CE and CEN Series (Fuji Clean) wastewater treatment systems. This request is predicated upon the ability of the Fuji Clean system to produce BOD5 and TSS levels below 10 mg/l, each.

The Division approves the request for reduced disposal field area and reduced separation distances as follows:

- 1. A minimum separation distance of 12 inches shall be maintained between the seasonal high groundwater table and the lowest elevation of the system's disposal field;
- 2. A minimum separation distance of 12 inches shall be maintained between bedrock and the lowest elevation of the system's disposal field;
- 3. Stone beds and trenches are allowed a 75 percent reduction in size, based upon the standard sizing requirements of the Rules;
- 4. Proprietary devices such as but not limited to plastic chambers and gravel-less pipe trenches are allowed a 50 percent reduction in size based upon the standard sizing requirements of the Rules, absent prohibitions by manufacturers;
- 5. Eljen GSF units may be used with Fuji Clean systems, but with no reduction in size; and
- 6. Maintenance agreement contracts must be included with all system installations. Terms and duration of the contracts shall be in accordance with Fuji Clean's company policies.

Because installation and maintenance has a significant effect on the working order of onsite sewage disposal systems, including their components, the Division makes no representation or guarantee as to the efficiency and/or operation of this system. Further, the Division strongly recommends that property owners enter into long term maintenance contracts with Fuji Clean, in accordance with Fuji Clean's company policies.

Page 2, Letter to Bennette D. Burkes, P. E.

Should you have any questions, please feel free to contact me.

Sincerely,

James A. Jacobsen

Project Manager, Webmaster Division of Environmental Health

Drinking Water Program

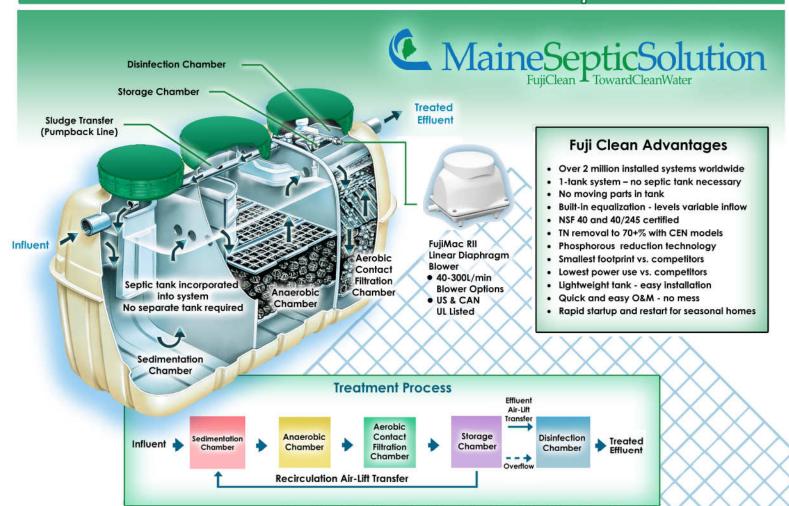
Engineering Review Team

e-mail: james.jacobsen@maine.gov

/jaj

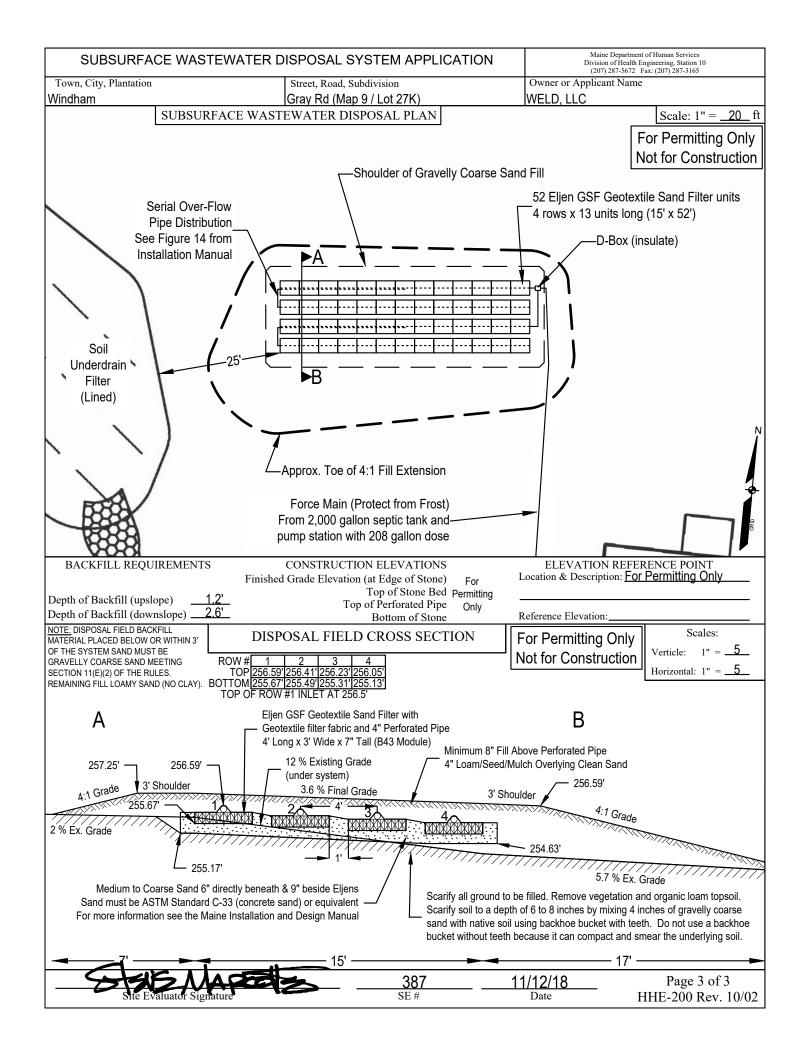
xc: File

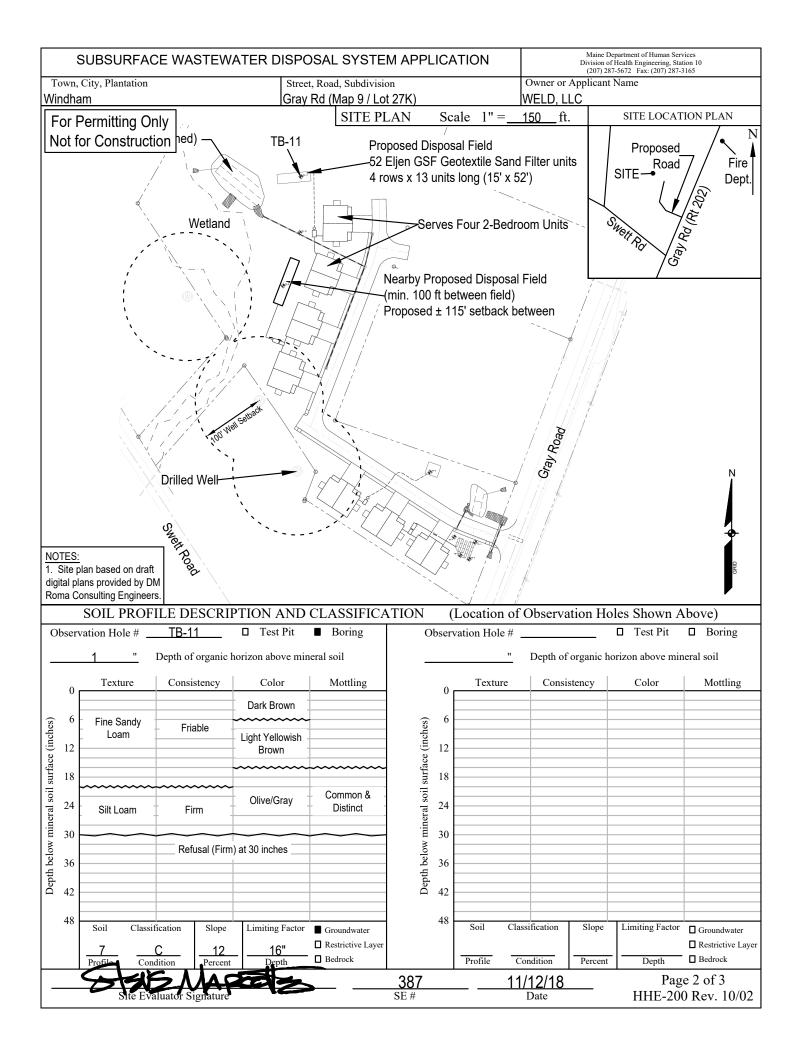
MODEL CE & CEN SERIES Technical Specification Sheet



Design Specification Table	CE Series BOD, TSS, TN							CEN Series BOD, TSS, Enhanced TN				
MODEL:	CE5	CE7	CE10	CE14	CE21	CE30	CE6KG	CEN5	CEN7	CEN10	CEN21	
Load Hydraulic (GPD)	360	540	720	1,000	1,900	2,700	6,000	360	540	720	1,900	
EFFLUENT (assumes domestic stren	gth influent):											
BOD – Effluent (mg/L)	10-20	10-20	10-20	10-20	10-20	10-20	10-20	10	10	10	10	
BOD (removal pounds/day)	.52	.73	1.04	1.46	2.08	3.12	6.93	.69	.97	1.38	2.9	
TSS (mg/L)	10-20	10-20	10-20	10-20	10-20	10-20	10-20	10	10	10	10	
TN (mg/L)	10-20	10-20	10-20	10-20	10-20	10-20	10-20	10	10	10	10	
BLOWER DETAIL:												
Blower Model	MAC80R	MAC80R	MAC100R	MAC100R	MAC150R	MAC200R	MAC200R (3)	MAC80R	MAC100R	MAC100R	MAC200	
Normal Pressure (kPa)	15	15	18	18	20	20	20	15	18	18	20	
Airflow Volume (CFM; L/Min.)	2.8 CFM 80 L/MIN	2.8 CFM 80 L/MIN	3.5 CFM 100 L/MIN	3.5 CFM 100 L/MINJ	5.3 CFM 150 L/MIN	7.0 CFM 200 L/MIN	21.0 CFM 600 L/MIN	2.8 CFM 80 L/MIN	3.5 CFM 100 L/MIN	3.5 CFM 100 L/MIN	7.0 CFM 200 L/MI	
Power Use (kWh/day)	1.1	1.1	1.6	1.6	2.4	3.4	10.2	1.1	1.6	1.6	3.4	
Weight (lbs.)	11	11	11	11	13	13	13 x 3	11	11	11	13	
Outlet Diameter (OD, inches)	0.70	0.70	0.70	0.70	1.0	1.0	1.0 x 3	.070	0.70	0.70	1.0	
TANK DETAIL:												
Material			Fibre	-Reinforced I	Plastic				Fibre-Reinfo	rced Plastic		
Height (inches)	61.8	65.7	73.6	77.4	81.3	87.2	87.2	65.7	73.6	77.4	87.2	
Length (inches)	85	95.7	98.8	118.9	152.8	183.7	434.7	95.7	98.8	118.9	183.7	
Width (inches)	43.7	49.2	56.7	68.9	72.4	78.3	115.3	49.2	56.7	68.9	78.3	
Weight (lbs.)	397	463	705	926	1,168	1,543	2,900	463	705	926	1,543	
Inlet Invert (inches)	49	53	61	62	65	71	67	53	61	62	71	
Outlet Invert (inches)	47	51	59	59.5	63	69	64	51	59	59.5	69	
Access Ports Quantity & Diameter (inches)	3@20"	2@20" 1@24"	2@20" 1@24"	2@20" 1@24"	2@20" 1@24"	2@20" 1@24"	4@24"x24" 3@24"x48"	2@20" 1@24"	2@20" 1@24"	2@20" 1@24"	2@20" 1@24"	
Tank Volume Total (gallons)	545	749	1,069	1,498	2.252	3,199	7,267	749	1.069	1,498	3,199	

		ASTEWATER DISP	OSA	AL SYSTEM	ΛAF	PPLICA	TION	Maine Department of Human Services Division of Health Engineering, 10 SHS (207) 287-5672 Fax: (207) 287-3165		
	PRÓPÉRTÝ	LOCATION		>> C	AUTI	ON: LPI AI	PPROVAL F	REQUIRED <<		
City, Town, or Plantation	Windham		Town	/City				Permit#		
Street or Road	Gray Rd (Map 9 / Lot 27K)	Date	Permit Issued/	/_	1	nitting Onl Construction	Double Fee Charged LI		
[· · · · · · · · · · · · · · · · · · ·						L.P.I. #		
	GIAY ROAD	Retirement Community		Local Plu	mbing I	nspector Signat	ure			
Name (last, first, MI)	RIAPPLICA	MI INFORMATION/////						Owner Town State		
WELD, LLC	The Subsurface Wastewater Disposal System shall not be installed until a Permit is issued by the Local Plumbing Inspector. This Permit shall									
Mailing Address of			•		•					
					•	al system in accordance				
	<u>vvinanam,</u>	ME 04062	with this application and the Maine Subsurface Wastewater Disposal Rules.							
Daytime Tel. #							Lot #	<u> </u>		
I state and acknowledge	e that the informa erstand that any f	ANT STATEMENT tion submitted is correct to the best of alsification is reason for the Department a Permit.			the inst		ed above and fo	und it to be in compliance tion. (1st) date approved		
Signa	ture of Owner or	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	,,,,	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		g Inspector Sigr	nature	(2nd) date approved		
	<u>////////</u>	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	,,,,	INFORMATION	///	<u>///////</u>	<u>///////</u>	<u>/////////////////////////////////////</u>		
TYPE OF APPLI		THIS APPLICATION	REQUI	RES				COMPONENTS ineered System		
■ 1. First Time Syst		■ 1. No Rule Variance						graywater & alt. toilet)		
☐ 2. Replacement S Type replaced:		☐ 2. First Time System Variance ☐ a. Local Plumbing Inspecto		val				specify:		
Year installed:		☐ b. State & Local Plumbing I					-	reatment Tank (only)		
□ 3. Expanded Syst	em	☐ 3. Replacement System Varia				☐ 5. Holding Tank,gallons ☐ 6. Non-engineered Disposal Field (only)				
a. <25% Expan		☐ a. Local Plumbing Inspecto☐ b. State & Local Plumbing I					arated Laundry	•		
□ b. >= 25% Expa		☐ 4. Minimum Lot Size Variance	•					red System (2000 gpd or more) ment Tank (only)		
4. Experimental S	-	☐ 5. Seasonal Conversion Perm	nit			-		sal Field (only)		
5. Seasonal Conv		DISDOSAL SYSTEM TO SE	D)/E		☐ 11. Pre-treatment, specify: ☐ 12. Miscellaneous Components					
SIZE OF PROP	ERII	DISPOSAL SYSTEM TO SE 1. Single Family Dwelling Unit, No		drooms:		mponents				
±11.4	□SQ. FT. ■ACRES	■ 2. Multiple Family Dwelling, No. o				TYPE OF WATER SUPPLY				
SHORELAND Z	ONING	3. Other:(specify)	[_]			☐ 1. Drilled W	☐ 1. Drilled Well ☐ 2. Dug Well ☐ 3. Private			
☐ Yes	■No	Current Use Seasonal Year F	Round	Undeveloped		4. Public	c ☐ 5. Other			
		/////DESIGN DÉTAILS (S	SÝSŤI	ÉM LAYOUT SH	ÍÓWŃ	Ý ÓN PÁGÍ	<u> </u>			
TREATMENT	TANK	DISPOSAL FIELD TYPE & SI		GARBAGE DI				DESIGN FLOW		
■ 1. Concrete		☐ 1. Stone Bed ☐ 2. Stone Trend	h	■ 1. No □ 2. Ye	es 🛮 3	B. Maybe	720			
■ a. Regular		■ 3. Proprietary Device		If Yes or Maybe, s	pecify	one below:		gallons per day \SED ON:		
b. Low Profile		☐ a. cluster array ☐ c. Linear	٨	a. multi-compar		tank	■ 1. Table 4	A (dwelling unit(s))		
☐ 2. Plastic ☐ 3. Other:		■ b. regular load □ d. H-20 loa □ 4. Other:	u	□ b tanks in s □ c. increase in ta		acity		IC (other facilities) CALCULATIONS		
CAPACITY: 2,	000 GAL	SIZE: 2496 sf stone bed equival	ent	d. Filter on Tank		-		ther facilities—		
SOIL DAT	-A	DISPOSAL FIELD SIZING		EFFLUENT/EJ						
PROFILE C	ONDITION	☐ 1. Medium2.6 sq. ft. / gpd		☐ 1. Not Required				4G (meter readings)		
	<u>C</u>		nd	☐ 2. May Be Requ	ired			WATER METER DATA		
at Observation Hole	# <u>TB-11</u>	2. MediumLarge 3.3 sq. f.t / g	pa	■ 3. Required				TITUDE AND LONGITUDE at center of disposal area		
Depth 16 "	F4	☐ 3. Large4.1 sq. ft. / gpd		Specify only for e	nainee	red systems:	LatN4:			
of Most Limiting Soil Factor ☐ 4. Extra Large5.0 sq. ft. / gpd				DOSE: 208	-	llons	Lon. <u>W7</u> if g.p.s. state	0 d 25 m 4.11 s e margin of error: 20'		
		//////////////////////////////////////	ÁLÚÁ	TOR STATÉMEI	 _					
I certify that on _	11/5/201			 		y and state	that the dat	a reported are accurate and		
that the propose	d system is i	n compliance with the State o	f Main	ne Subsurface W	astev	vater Dispo	sal Rules (1	10-144A CMR 241).		
- Su-	515/	MACE		387		-	1/12/18	•		
S	ite Evaluato			SE#		<u></u>	Date	_		
		_		(207) 020 (aenr)	rootto@-	ummitaccona com		
	<u>ephen B. I</u> e Evaluator	<u>VIACCOTTE</u> Name Printed		(207) 939-2				summitgeoeng.com ail Address		
Designed with S				Telephone Nu	mber		⊏ina	Page 1 of 3		
		ons from the design should be	e confi	irmed with the Si	te Ev	aluator.		HHE-200 Rev. 08/2011		







Attachment 3

Wetland Nitrate-Nitrogen Removal Calculations



WETLAND NITROGEN REMOVAL CALCULATIONS

Date: 11/16/2018

Project: Gray Road Retirement Community

Project #: 18323

Summit Personnel: Stephen B. Marcotte, C.G., L.S.E.

Site Location: Gray Road, Windham, Maine

Assumptions: Wetlands remove nitrogen through both plant uptake and microbial activity (denitrification). The capacity of

planted and unplanted wetlands to remove nitrogen at concentration similar to those in treated septic system effluent show nitrogen removal rates in excess of 95% for planted wetlands and removal rates of 25% to 36% in unplanted (natural) wetlands. Assuming a 25% nitrate removal rate wetland area nitrate removal

rate of 0.0825 grams per square.

The nitrate-nitrogen plumes from subsurface wastewater disposal fields C and D will flow to and through an on-site wetland area approximately 21,000 square feet in size. Approximately 16,000 square feet of wetland is located along the anticipated flow pathway through this wetland prior to crossing the property boundary. The flow pathway length is approximately 200 feet.

Disposal Field C has a design flow of 720 gallons per day, a Fuji Clean CE10 advanced treatment unit, and treated effluent discharged to the disposal field will have a concentration of 20 mg-N/L total nitrogen.

Disposal Field D has a design flow of 720 gallons per day, no advanced treatment, and treated effluent discharged to the disposal field will have a concentration of 40 mg-N/L nitrate-nitrogen.

The combined flow from both disposal fields is 1440 gallons per day and the weighted average concentration of nitrate-nitrogen is 30 mg-N/L.

Calculations: Nitrogen Loading from Disposal Fields

1,440 gallons per day 5451.0 liters/day 30 mg-N/L 163529.7 mg-N/day 163.5 grams-N/day

Wetland Size & Nitrogen Removal Rate

16,000 square feet 1469.2 sq. meters 0.0825 grams / sq. meters / day

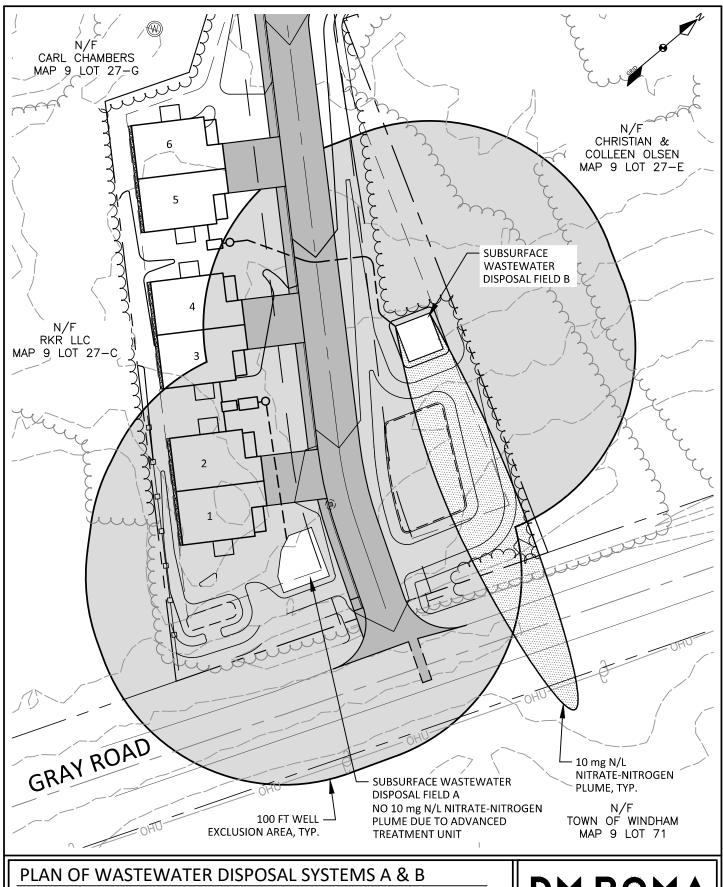
Results

121.21 grams / day removed 42.32 grams / day remaining

7.76 mg/L in groundwater after wetland treatment

References:

Lin, et. al. (2002), Effects of macrophytes and external carbon sources on nitrate removal from groundwater in constructed wetlands. Environmental Pollution, v. 119, pp. 413-420.



GRAY ROAD RETIREMENT COMMUNITY WINDHAM, MAINE

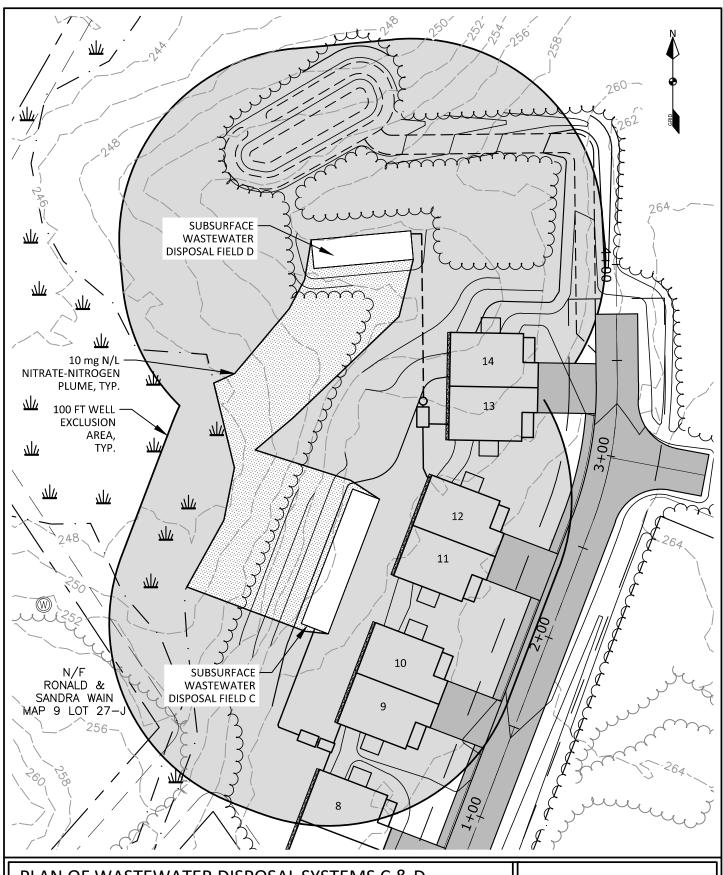
FOR RECORD OWNER: WELD,LLC

PO BOX 1361 WINDHAM, MAINE 04062 SCALE: 1"=50' DATE: 11-13-2018 JOB NUMBER: 17070

DM ROMA

CONSULTING ENGINEERS

P.O. BOX 1116 WINDHAM, ME 04062 (207) 310 - 0506



PLAN OF WASTEWATER DISPOSAL SYSTEMS C & D

GRAY ROAD RETIREMENT COMMUNITY WINDHAM, MAINE

FOR RECORD OWNER: WELD,LLC

PO BOX 1361 WINDHAM, MAINE 04062

SCALE: 1"=50' DATE: 11-13-2018 JOB NUMBER: 17070 DM ROMA

CONSULTING ENGINEERS

P.O. BOX 1116 WINDHAM, ME 04062 (207) 310 - 0506



STORMWATER MANAGEMENT REPORT

GRAY ROAD RETIREMENT COMMUNITY WINDHAM, MAINE

A. Narrative

Weld, LLC is proposing to develop a 11.4-acre parcel off Gray Road (Rt. 202) in Windham. The project site is located on Lot 27K on the Town of Windham Assessors Map 9 and is located in the Farm Residential District and Retirement Community and Care Facility Overlay District.

This proposed retirement community development will consist of seven (7) residential duplex buildings, totaling twelve (14) residential units including the construction of approximately 800 linear feet of paved roadway, utilities and stormwater infrastructure. In general, the site drains either easterly to Gray Road or northwesterly to a drainage wetland draining northerly along the property boundary. Runoff along Gray Road drains across the street, eventually discharging to Black Brook. Runoff collected in the wetlands along the northwesterly property line are eventually tributary to the Pleasant River. Both waterbodies are ultimately tributary to the Presumpscot River.

B. Alterations to Land Cover

The 11.4-acre parcel consists of undeveloped woodland. The proposed development will generate approximately 43,494 square feet (0.99 \pm acres) of new impervious area consisting of the seven (7) structures (duplexes ~ 17,442 square feet (0.40 \pm acres)), and 26,052 square feet (0.60 \pm acres) of new roadway and driveways. An additional 62,520 square feet (1.44 \pm acres) of proposed lawn and landscaping will generate a total site developed area of approximately 106,014square feet (2.43 \pm acres).

Since the project site will not generate more than one (1) acre of new impervious surface or five (5) acres of new developed area, a Stormwater Permit will not be required from the Maine Department of Environmental Protection (MDEP). The site will be required to obtain an MDEP Stormwater Permit by Rule prior to construction since it will generate more than one (1) acre of land disturbance. The project will be reviewed by the Town of Windham as a Major Subdivision.

The site is generally moderate to steeply sloped (5%-33%), with a portion of the site in the vicinity of the proposed roadway intersection with Gray Road being relatively flat (2%-5%). The site drains in two predominant drainage patterns; with the northwesterly portion of the

site draining to the northwesterly property limits, and the southeasterly portion of the site draining to the southwest and into a drainage swale along Gray Road. The onsite soils are primarily Belgrade very fine sandy loam, Lamoine silt loam, Lyman-Turnbridge complex, Paxton fine sandy loam, and Woodbridge fine sandy loam as identified on the Medium Intensity Soil Maps for Cumberland County, Maine published by the Natural Resources Conservation Service. The soils within the proposed development are in the hydrologic soil group "B", "C". "C/D" and "D".

The soils boundaries and hydrologic soils group (HSG) designations are indicated on the Watershed Maps and the Medium Intensity Soils Map has been included as Attachment 1 of this report.

C. Methodology and Modeling Assumptions

The proposed stormwater management system has been designed utilizing Best Management Practices to maintain existing drainage patterns while providing stormwater quality improvement measures. The goal of the storm drainage system design is to remove potential stormwater pollutants from runoff generated by the development while providing attenuation of the peak rates of runoff leaving the site. The method utilized to predict the surface water runoff rates in this analysis is a computer program entitled HydroCAD, which is based on the same methods that were originally developed by the U.S. Department of Agriculture (USDA), Natural Resources Conservation Service, and utilized in the TR-20 modeling program. Peak rates of runoff are forecasted based upon land use, hydrologic soil conditions, vegetative cover, contributing watershed area, time of concentration, rainfall data, storage volumes of detention basins and the hydraulic capacity of structures. The computer model predicts the amount of runoff as a function of time, with the ability to include the attenuation effect due to dams, lakes, large wetlands, floodplains and constructed stormwater management basins. The input data for rainfalls with statistical recurrence frequencies of 2-, 10- and 25 years was obtained from Appendix H of the MDEP, Chapter 500 Stormwater Management, last revised in 2015. The National Weather Service developed four synthetic storm types to simulate rainfall patterns around the country. For analysis in Cumberland County, Maine, the type III rainfall pattern with a 24-hour duration is appropriate.

D. Basic Standards

The project is required by the Town and the MDEP to provide permanent and temporary Erosion Control Best Management Practices. These methods are incorporated into the project design and outlined in detail in the plan set.

E. General Standard

The Town of Windham requires the entire project to meet the General Standards outlined in the MDEP Chapter 500 to provide water quality treatment for no less than 95% of the new impervious surface and 80% of the total developed area associated with the project.

To provide the required stormwater treatment for the development, two (2) underdrained filter basins, one (1) bio-retention cell, and roofline dripedges along the residential buildings will be constructed as part of the site's stormwater infrastructure. As a result of the proposed stormwater infrastructure the project provides water quality treatment for over 96% of the equivalent new impervious and for over 83% of the new developed areas. Calculations can be found on the Watershed Maps and enclosed as Attachment 2 in this report.

F. Flooding Standard

The Town of Windham Land Use Ordinance requires the project to detain, retain or result in the infiltration of stormwater from the 24-hour storms of the 2-year, 10-year and 25-year frequencies such that the peak flows of stormwater from the project site do not exceed the peak flows of stormwater prior to undertaking the project. To maintain these rates, two (2) underdrained filter basins, and one (1) bio-retention cell have been proposed as part of the stormwater infrastructure.

The proposed project design has been modeled to evaluate and analyze the stormwater runoff characteristics of the site prior to construction of the project and upon completion of all proposed construction activities. Since the project site is not being completely utilized, the study was performed on the area of development. The first study point (SP-1) is located along the southeasterly portion of the site, to the north of the proposed roadway intersection with Gray Road. The second study point (SP-2) is along the westerly property limit which discharged and then conveyed overland in an existing natural drainage channel.

The following table summarizes the analysis prepared for this stormwater management report:

	Table 1 – Peak Rates of Stormwater Runoff											
Study Point	2-Year (cfs) 10-Year (cfs) 25-Year (cfs)											
	Pre	Post	Pre	Post	Pre	Post						
SP-1	0.55	0.55	1.28	1.23	1.98	1.82						
SP-2	SP-2 3.71 3.36 7.42 6.90 10.67 10											

As illustrated by the table above, the proposed BMP's as incorporated in the project's storm water design, effectively maintains or reduces the peak flow at all study points, during all storms events.

The watershed maps showing pre-development and post-development drainage patterns are included in the plan set and the computations performed with the HydroCAD software program are included as Attachment 3 of this report.

G. Maintenance of common facilities or property

The applicant will be responsible for the maintenance of the stormwater facilities until a homeowners' association is created. Enclosed within this submission is an Inspection, Maintenance and Housekeeping Plan for the project.

Prepared by:

DM ROMA CONSULTING ENGINEERS

J.P. Connolly

Senior Project Engineer

Jayson R. Haskell P.E.

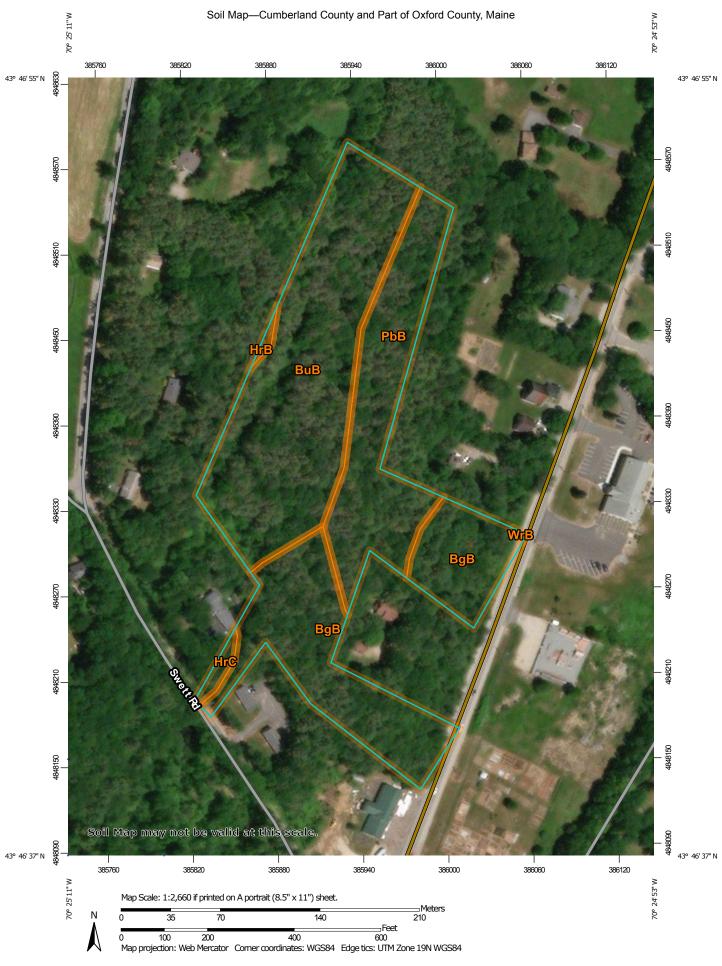
Southern Maine Regional Manager

ayson R. Haskell

AHHIIII

ATTACHMENT 1

MEDIUM INTENSITY SOILS MAP



MAP LEGEND

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Water Features

Transportation

Background

Spoil Area

Stony Spot

Wet Spot

Other

Rails

US Routes

Major Roads

Local Roads

Very Stony Spot

Special Line Features

Streams and Canals

Interstate Highways

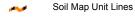
Aerial Photography

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons



Soil Map Unit Points

Special Point Features

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Landfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

→ Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

The soil surveys that comprise your AOI were mapped at 1:24.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

MAP INFORMATION

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Cumberland County and Part of Oxford County, Maine

Survey Area Data: Version 13, Sep 11, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Apr 29, 2012—Jun 26, 2016

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
BgB	Belgrade very fine sandy loam, 0 to 8 percent slopes	3.6	31.4%
BuB	Lamoine silt loam, 3 to 8 percent slopes	5.0	43.4%
HrB	Lyman-Tunbridge complex, 0 to 8 percent slopes, rocky	0.0	0.3%
HrC	Lyman-Tunbridge complex, 8 to 15 percent slopes, rocky	0.1	1.1%
PbB	Paxton fine sandy loam, 3 to 8 percent slopes	2.7	23.7%
WrB	Woodbridge fine sandy loam, 0 to 8 percent slopes	0.0	0.1%
Totals for Area of Interest		11.5	100.0%

ATTACHMENT 2

STORMWATER TREATMENT CALCULATIONS

Stormwater Treatment Table

Gray Road Retirement Community

		New Driveway							New Impervious	New Landscaped	
		and Road			Existing/Offsite	Existing/Offsite	Existing		Area Treated In	Area Treated In	
	Total Watershed	Total Watershed Impervious Area	New Building	New Landscaped	Impervious Area	Landscaped Area	Undeveloped	Treatment	Treatment Device	Treatment Device	Treatment
	Area (SF)	(SF)	Area (SF)	Area (SF)	(SF)**	(SF)**	Area (SF)	Provided	(SF)	(SF)	Device
WS-11	32,325	13,090	8,566	10,670	0	0	0	Yes	21,656	10,670	FB1
WS-12	25,130	6,451	0	16,616	0	0	2,062	Yes	6,451	16,616	FB1
WS-13	095'6	529	0	5,974	0	0	136	Yes	259	5,974	BR1
WS-13 (1	WS-13 (treated by drip edge) *	re) *	3,192					Yes	3,192	0	DripEdge
WS-21	21,056	4,655	1,428	12,988	0	0	1,985	Yes	6,083	12,988	FB2
WS-1	15,362	1,289	0	1,861	3,571	3,306	5,335	oN	0	0	None
WS-2	159,814	308	0	14,411	0	0	140,839	ON	0	0	None
WS-2 (tr	WS-2 (treated by drip edge)	* (:	4,256					Yes	4,256	0	DripEdge
Total		26,052	17,442	62,520					41,897	46,248	
* * * * *	Ictai IIcda agaibli	12 roofling drip od	202 04+ 200 02	** Il now building chall inchall a gradies adar a process of the character of the character of the character of the character of the building important or a character of the building in the	d+ rof + com+cost object		The Thirt	acaci a'sailai	Lock. loci of core official	, '4+ 	

'All new buildings shall install a roofline drip edge along the rear of the structure to provide treatment for the rooftop impervious surface. The building's impervious area is included in the

** The project is not taking credit for the Existing or Offsite impervious and landscaped areas, but are included in the BMP sizing calculations for each treatment device watershed and overall treatment calculations below, but not included in the BMP sizing calculations for each treatment device

43,494 sf 41,320 sf 41,897 sf Impervious Area Requiring Treatment (95%) = Impervious Area Treatment Provided = New Impervious Area =

96% New Impervious Area Treated

106,014 sf 84,811 sf Developed Area Requiring Treatment (80%) = New Developed Area =

Developed Area Treatment Provided =

83% New Developed Area Treated 88,145 sf

Filter Basin FB-1

Tributary Impervious Area 28,107 sf (WS-11 & WS-12 Impervious Area)
Tributary Landscaped Area 27,286 sf (WS-11 & WS-12 Landscaped Area)

Water Quality Volume (WQV) Calculation

WQV (Required) = 1.0"xImpervious Area + 0.4"xLandscaped Area

WQV (Required) = 3,252 cf

Stage Storage Volume

Elevation Area (sf) Storage (cf)

253.75 2,103 0 257.25 5,037 11,846

Outlet Elevation = 256.40

Storage Volume Provided = 4,045 cf > Required

Filter Bottom Calculation

Filter Area (Required) = 5%xImpervious Area + 2%xLandscaped Area

Filter Area Required = 1,951 sf

Filter Area Provided = 2,103 sf > Required

Filter Basin FB-2

Water Quality Volume (WQV) Calculation

WQV (Required) = 1.0"xImpervious Area + 0.4"xLandscaped Area

WQV (Required) = 940 cf

Stage Storage Volume

Elevation Area (sf) Storage (cf)

250.5 702 0 253 2,008 3,305

Outlet Elevation = 252.00

Storage Volume Provided= 1,588 cf > Required

Filter Bottom Calculation

Filter Area (Required) = 5%xImpervious Area + 2%xLandscaped Area

Filter Area Required = 564 sf

Filter Area Provided = 702 sf > Required

Bioretention Cell BR-1

Tributary Impervious Area= 259 sf (WS-13 Impervious Area)
Tributary Landscaped Area= 5,974 sf (WS-13 Landscaped Area)

Water Quality Volume (WQV) Calculation

WQV (Required) = 1.0"xImpervious Area + 0.4"xLandscaped Area

WQV (Required) = 221 cf

Stage Storage Volume

Elevation Area (sf) Storage (cf)

256.9 205 0 257.4 309 128

Storage From Filter Media (1/3 Filter Volume)= 103 cf
Outlet Elevation = 257.40
Storage Volume Above Media= 128 cf

Total Storage Volume Provided= 231 cf > Required

Filter Bottom Calculation

Filter Area (Required) = 7%xImpervious Area + 3%xLandscaped Area

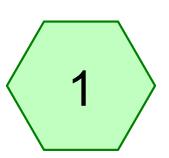
Filter Area Required = 197 sf

Filter Area Provided = 205 sf > Required

ATTACHMENT 3

HYDROCAD OUTPUT













Routing Diagram for 17070 - PRE
Prepared by {enter your company name here}, Printed 11/16/2018
HydroCAD® 10.00-22 s/n 09237 © 2018 HydroCAD Software Solutions LLC

17070 - PRE

Type III 24-hr 2-Year Rainfall=3.10", Ia/S=0.10

Prepared by {enter your company name here}

Printed 11/16/2018

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Page 2

Time span=0.00-48.00 hrs, dt=0.10 hrs, 481 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment1: Runoff Area=58,625 sf 4.86% Impervious Runoff Depth=0.68"

Flow Length=345' Tc=29.5 min CN=61 Runoff=0.55 cfs 3,341 cf

Subcatchment2: Runoff Area=204,250 sf 0.00% Impervious Runoff Depth=1.16"

Flow Length=447' Tc=24.4 min CN=73 Runoff=3.71 cfs 19,734 cf

Total Runoff Area = 262,875 sf Runoff Volume = 23,075 cf Average Runoff Depth = 1.05" 98.92% Pervious = 260,024 sf 1.08% Impervious = 2,851 sf

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Page 3

Summary for Subcatchment 1:

Runoff = 0.55 cfs @ 12.46 hrs, Volume= 3,341 cf, Depth= 0.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 2-Year Rainfall=3.10", Ia/S=0.10

	Α	rea (sf)	CN	Description		
*		2,851	98	Gray Road	- Ex. Pave	d roads, HSG B
*		912	96	Ex. drivewa	y - Gravel	surface, HSG B
		54,862	58	Woods/gras	ss comb., C	Good, HSG B
		58,625	61	Weighted A	verage	
		55,774	59	95.14% Pei	rvious Area	
		2,851	98	4.86% Impe	ervious Are	a
	Tc	Length	Slope	e Velocity	Capacity	Description
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	25.5	150	0.0304	0.10		Sheet Flow, Seg A to B
						Woods: Light underbrush n= 0.400 P2= 3.10"
	4.0	195	0.0270	0.82		Shallow Concentrated Flow, Seg B to C
						Woodland Kv= 5.0 fps
	29.5	345	Total			

Summary for Subcatchment 2:

Runoff = 3.71 cfs @ 12.36 hrs, Volume= 19,734 cf, Depth= 1.16"

A	rea (sf)	CN [Description		
	48,931	58 V	Voods/gras	ss comb., G	Good, HSG B
	35,748				Good, HSG C
1	19,571	79 \	Voods/gras	ss comb., G	Good, HSG D
2	04,250		Veighted A		
2	04,250	73 1	00.00% Pe	ervious Are	a
_		٥.			—
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
22.3	150	0.0427	0.11		Sheet Flow, Seg A to B
					Woods: Light underbrush n= 0.400 P2= 3.10"
1.1	39	0.0153	0.62		Shallow Concentrated Flow, Seg B to C
					Woodland Kv= 5.0 fps
0.7	85	0.1879	2.17		Shallow Concentrated Flow, Seg C to D
					Woodland Kv= 5.0 fps
0.3	173	0.0404	11.41	272.75	, <u> </u>
					Area= 23.9 sf Perim= 25.6' r= 0.93'
					n= 0.025 Earth, clean & winding
24.4	447	Total			

17070 - PRE

Type III 24-hr 10-Year Rainfall=4.60", la/S=0.10

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Page 4

Time span=0.00-48.00 hrs, dt=0.10 hrs, 481 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment1: Runoff Area=58,625 sf 4.86% Impervious Runoff Depth=1.52"

Flow Length=345' Tc=29.5 min CN=61 Runoff=1.28 cfs 7,402 cf

Subcatchment2: Runoff Area=204,250 sf 0.00% Impervious Runoff Depth=2.26"

Flow Length=447' Tc=24.4 min CN=73 Runoff=7.42 cfs 38,414 cf

Total Runoff Area = 262,875 sf Runoff Volume = 45,815 cf Average Runoff Depth = 2.09" 98.92% Pervious = 260,024 sf 1.08% Impervious = 2,851 sf 17070 - PRE

Type III 24-hr 25-Year Rainfall=5.80", Ia/S=0.10

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Page 6

Time span=0.00-48.00 hrs, dt=0.10 hrs, 481 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

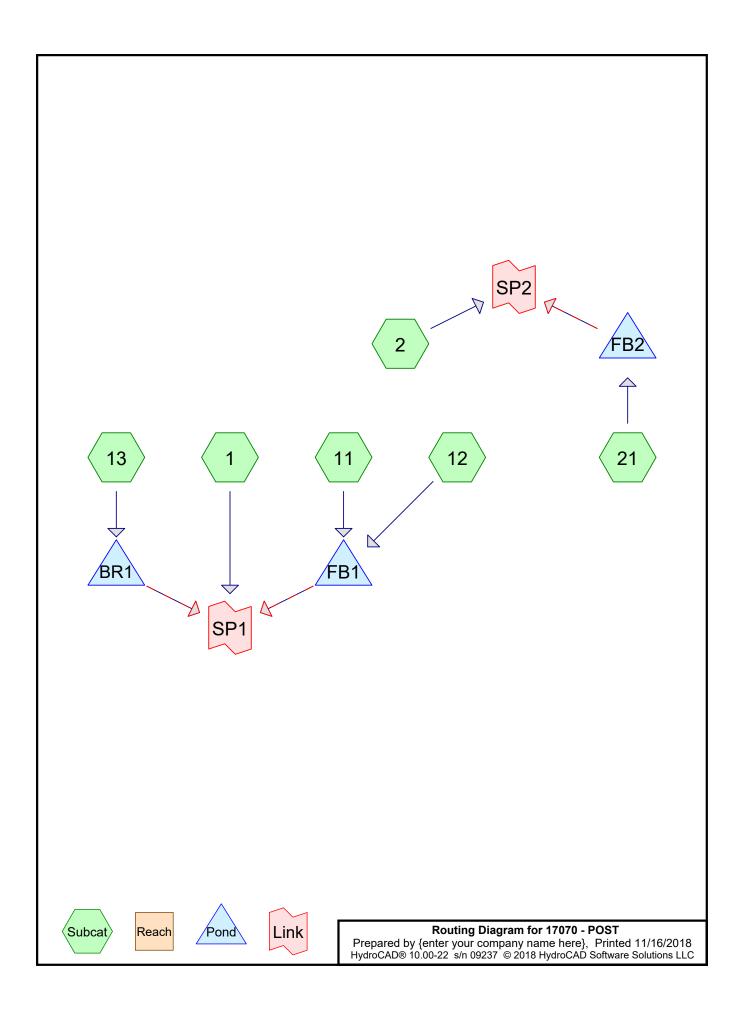
Subcatchment1: Runoff Area=58,625 sf 4.86% Impervious Runoff Depth=2.31"

Flow Length=345' Tc=29.5 min CN=61 Runoff=1.98 cfs 11,261 cf

Subcatchment2: Runoff Area=204,250 sf 0.00% Impervious Runoff Depth=3.23"

Flow Length=447' Tc=24.4 min CN=73 Runoff=10.67 cfs 54,978 cf

Total Runoff Area = 262,875 sf Runoff Volume = 66,239 cf Average Runoff Depth = 3.02" 98.92% Pervious = 260,024 sf 1.08% Impervious = 2,851 sf



17070 - POST

Type III 24-hr 2-Year Rainfall=3.10", la/S=0.10

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Page 2

Time span=0.00-48.00 hrs, dt=0.10 hrs, 481 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: Runoff Area=15,362 sf 31.64% Impervious Runoff Depth=1.07"

Flow Length=133' Slope=0.0356'/' Tc=14.5 min CN=71 Runoff=0.31 cfs 1,369 cf

Subcatchment2: Runoff Area=159,814 sf 2.86% Impervious Runoff Depth=1.30"

Flow Length=447' Tc=24.4 min CN=76 Runoff=3.30 cfs 17,374 cf

Subcatchment 11: Runoff Area=32,325 sf 66.99% Impervious Runoff Depth=1.89"

Flow Length=551' Tc=12.3 min CN=86 Runoff=1.25 cfs 5,091 cf

Subcatchment 12: Runoff Area=25,130 sf 25.67% Impervious Runoff Depth=1.07"

Flow Length=473' Tc=7.1 min CN=71 Runoff=0.63 cfs 2,239 cf

Subcatchment 13: Runoff Area=9,560 sf 36.10% Impervious Runoff Depth=1.21"

Flow Length=316' Tc=22.9 min CN=74 Runoff=0.19 cfs 961 cf

Subcatchment 21: Runoff Area=21,056 sf 28.89% Impervious Runoff Depth=1.63"

Flow Length=384' Tc=12.2 min CN=82 Runoff=0.70 cfs 2,868 cf

Total Runoff Area = 263,247 sf Runoff Volume = 29,902 cf Average Runoff Depth = 1.36" 82.12% Pervious = 216,182 sf 17.88% Impervious = 47,065 sf Prepared by {enter your company name here}

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Page 3

Summary for Subcatchment 1:

Runoff = 0.31 cfs @ 12.22 hrs, Volume= 1,369 cf, Depth= 1.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 2-Year Rainfall=3.10", la/S=0.10

	Д	rea (sf)	CN I	Description			
*		3,571	98 (Gray Road	- Ex. Paved	d roads, HSG B	
		5,167	61 >	>75% Gras	s cover, Go	ood, HSG B	
		1,289	98 I	Paved road	s w/curbs 8	& sewers, HSG B	
		896	55 \	Noods, Go	od, HSG B		
_		4,439	58 I	Meadow, no	on-grazed,	HSG B	
		15,362	71 \	Weighted A	verage		
		10,502	6	88.36% Per	vious Area		
		4,860	(31.64% Impervious Area			
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	14.5	133	0.0356	0.15		Sheet Flow, Seg A to B	
						Grass: Dense n= 0.240	P2= 3.10"

Summary for Subcatchment 2:

Runoff = 3.30 cfs @ 12.35 hrs, Volume= 17,374 cf, Depth= 1.30"

	Area (sf)	CN	Description
	5,517	58	Woods/grass comb., Good, HSG B
	17,318	72	Woods/grass comb., Good, HSG C
	100,408	79	Woods/grass comb., Good, HSG D
	8,024	58	Meadow, non-grazed, HSG B
	9,573	78	Meadow, non-grazed, HSG D
	10,216	61	>75% Grass cover, Good, HSG B
	1,312	74	>75% Grass cover, Good, HSG C
	2,882	80	>75% Grass cover, Good, HSG D
*	4,256	98	Proposed Roofs w/ drip edge
*	308	98	Proposed misc. hardscape, Paved
	159,814 155,250 4,564	76	Weighted Average 97.14% Pervious Area 2.86% Impervious Area

17070 - POST

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Page 4

	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	22.3	150	0.0427	0.11		Sheet Flow, Seg A to B
						Woods: Light underbrush n= 0.400 P2= 3.10"
	1.1	39	0.0153	0.62		Shallow Concentrated Flow, Seg B to C
						Woodland Kv= 5.0 fps
	0.7	85	0.1879	2.17		Shallow Concentrated Flow, Seg C to D
						Woodland Kv= 5.0 fps
	0.3	173	0.0404	11.41	272.75	Channel Flow, Seg D to E
						Area= 23.9 sf Perim= 25.6' r= 0.93'
						n= 0.025 Earth, clean & winding
	24.4	447	Total			

Summary for Subcatchment 11:

Runoff = 1.25 cfs @ 12.19 hrs, Volume= 5,091 cf, Depth= 1.89"

	Α	rea (sf)	CN D	escription		
		8,566	98 F	Roofs, HSG	ВВ	
*		13,090				eways & misc. hardscape, Paved
		10,567	61 >	75% Gras	s cover, Go	ood, HSG B
		102	74 >	75% Gras	s cover, Go	ood, HSG C
		32,325	86 V	Veighted A	verage	
		10,669	3	3.01% Per	vious Area	
		21,656	6	6.99% Imp	pervious Are	ea
	Tc	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	10.7	57	0.0138	0.09		Sheet Flow, Seg A to B
						Grass: Dense n= 0.240 P2= 3.10"
	0.2	15	0.0296	1.08		Sheet Flow, Seg B to C
						Smooth surfaces n= 0.011 P2= 3.10"
	0.6	168	0.0109	4.72	29.09	, , , , , , , , , , , , , , , , , , ,
						Bot.W=0.08' D=0.50' Z= 48.0 & 1.0 '/' Top.W=24.58'
						n= 0.013 Asphalt, smooth
	0.7	281	0.0146	6.36	7.81	1 , , ,
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.013 Corrugated PE, smooth interior
	0.1	30	0.0070	4.40	5.40	Pipe Channel, Seg E to F
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
_						n= 0.013 Corrugated PE, smooth interior
	12.3	551	Total			

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Summary for Subcatchment 12:

Runoff = 0.63 cfs @ 12.12 hrs, Volume= 2,239 cf, Depth= 1.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.10 hrs Type III 24-hr 2-Year Rainfall=3.10", Ia/S=0.10

A	rea (sf)	CN [Description		
	6,451	98 F	Paved road	s w/curbs 8	& sewers, HSG B
	2,062	55 \	Noods, Go	od, HSG B	
	14,559	61 >	>75% Gras	s cover, Go	ood, HSG B
	2,058	74 >	>75% Gras	s cover, Go	ood, HSG C
	25,130	71 \	Weighted A	verage	
	18,679	7	74.33% Pei	vious Area	
	6,451	2	25.67% Imp	ervious Are	ea
			_		
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.6	52	0.0383	0.13		Sheet Flow, Seg A to B
					Grass: Dense n= 0.240 P2= 3.10"
0.5	421	0.0196	14.35	139.88	Trap/Vee/Rect Channel Flow, Seg B to C
					Bot.W=2.00' D=1.50' Z= 3.0 '/' Top.W=11.00'
					n= 0.013 Asphalt, smooth
7.1					

Summary for Subcatchment 13:

Runoff = 0.19 cfs @ 12.33 hrs, Volume= 961 cf, Depth= 1.21"

	Α	rea (sf)	CN	Description		
		5,973	61	>75% Gras	s cover, Go	ood, HSG B
		259	98	Paved road	s w/curbs &	& sewers, HSG B
		3,192	98	Roofs, HSC	βB	
_		136	58	Woods/gras	ss comb., G	Good, HSG B
		9,560	74	Weighted A	verage	
		6,109		63.90% Pe	rvious Area	
		3,451		36.10% lm	pervious Ar	ea
	Тс	Length	Slope	•	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	22.5	150	0.0150	0.11		Sheet Flow, Seg A to B
						Grass: Dense n= 0.240 P2= 3.10"
	0.4	166	0.0285	7.14	35.72	, , , , , , , , , , , , , , , , , , ,
						Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00'
_						n= 0.025 Earth, clean & winding
	22.9	316	Total			

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Summary for Subcatchment 21:

Runoff = 0.70 cfs @ 12.19 hrs, Volume= 2,868 cf, Depth= 1.63"

	A	Area (sf)	CN	Description		
		1,428	98	Roofs, HSG	ВВ	
*		3,593	98	Proposed S	ub. Road, ¡	paved roads w/curbs
*		1,062	98	Proposed d	rivewasy &	misc. hardscape, paved
		630	61	>75% Gras	s cover, Go	ood, HSG B
		6,586	74	>75% Gras	s cover, Go	ood, HSG C
		5,772	80	>75% Gras	s cover, Go	ood, HSG D
		1,985	72	Woods/gras	ss comb., G	Good, HSG C
		21,056	82	Weighted A	verage	
		14,973		71.11% Per	vious Area	
		6,083		28.89% lmp	pervious Are	ea
	_					—
	Tc		Slope		Capacity	Description
	(min)	(feet)	(ft/ft		(cfs)	
	11.0	60	0.0144	0.09		Sheet Flow, Seg A to B
						Grass: Dense n= 0.240 P2= 3.10"
	0.4	24	0.0166	0.94		Sheet Flow, Seg B to C
						Smooth surfaces n= 0.011 P2= 3.10"
	0.3	49	0.0188	2.78		Shallow Concentrated Flow, Seg C to D
						Paved Kv= 20.3 fps
	0.1	21	0.1454	6.14		Shallow Concentrated Flow, Seg D to E
						Unpaved Kv= 16.1 fps
	0.4	230	0.0429	9.20	89.67	Trap/Vee/Rect Channel Flow, Seg E to F
						Bot.W=2.00' D=1.50' Z= 3.0 '/' Top.W=11.00'
						n= 0.030 Earth, grassed & winding
	12.2	384	Total			

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Type III 24-hr 10-Year Rainfall=4.60", la/S=0.10

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Time span=0.00-48.00 hrs, dt=0.10 hrs, 481 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: Runoff Area=15,362 sf 31.64% Impervious Runoff Depth=2.12"

Flow Length=133' Slope=0.0356 '/' Tc=14.5 min CN=71 Runoff=0.64 cfs 2,718 cf

Subcatchment2: Runoff Area=159,814 sf 2.86% Impervious Runoff Depth=2.47"

Flow Length=447' Tc=24.4 min CN=76 Runoff=6.37 cfs 32,846 cf

Subcatchment 11: Runoff Area=32,325 sf 66.99% Impervious Runoff Depth=3.25"

Flow Length=551' Tc=12.3 min CN=86 Runoff=2.12 cfs 8,745 cf

Subcatchment 12: Runoff Area=25,130 sf 25.67% Impervious Runoff Depth=2.12"

Flow Length=473' Tc=7.1 min CN=71 Runoff=1.27 cfs 4,446 cf

Subcatchment 13: Runoff Area=9,560 sf 36.10% Impervious Runoff Depth=2.33"

Flow Length=316' Tc=22.9 min CN=74 Runoff=0.37 cfs 1,853 cf

Subcatchment 21: Runoff Area=21,056 sf 28.89% Impervious Runoff Depth=2.92"

Flow Length=384' Tc=12.2 min CN=82 Runoff=1.25 cfs 5,120 cf

Total Runoff Area = 263,247 sf Runoff Volume = 55,727 cf Average Runoff Depth = 2.54" 82.12% Pervious = 216,182 sf 17.88% Impervious = 47,065 sf

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Type III 24-hr 25-Year Rainfall=5.80", Ia/S=0.10

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Time span=0.00-48.00 hrs, dt=0.10 hrs, 481 points x 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: Runoff Area=15,362 sf 31.64% Impervious Runoff Depth=3.07"

Flow Length=133' Slope=0.0356'/' Tc=14.5 min CN=71 Runoff=0.93 cfs 3,927 cf

Subcatchment2: Runoff Area=159,814 sf 2.86% Impervious Runoff Depth=3.48"

Flow Length=447' Tc=24.4 min CN=76 Runoff=9.00 cfs 46,349 cf

Subcatchment 11: Runoff Area=32,325 sf 66.99% Impervious Runoff Depth=4.37"

Flow Length=551' Tc=12.3 min CN=86 Runoff=2.83 cfs 11,783 cf

Subcatchment 12: Runoff Area=25,130 sf 25.67% Impervious Runoff Depth=3.07"

Flow Length=473' Tc=7.1 min CN=71 Runoff=1.85 cfs 6,424 cf

Subcatchment 13: Runoff Area=9,560 sf 36.10% Impervious Runoff Depth=3.31"

Flow Length=316' Tc=22.9 min CN=74 Runoff=0.53 cfs 2,639 cf

Subcatchment 21: Runoff Area=21,056 sf 28.89% Impervious Runoff Depth=4.01"

Flow Length=384' Tc=12.2 min CN=82 Runoff=1.71 cfs 7,028 cf

Total Runoff Area = 263,247 sf Runoff Volume = 78,150 cf Average Runoff Depth = 3.56" 82.12% Pervious = 216,182 sf 17.88% Impervious = 47,065 sf

ATTACHMENT 4

INSPECTION, MAINTENANCE & HOUSEKEEPING PLAN



INSPECTION, MAINTENANCE, AND HOUSEKEEPING PLAN

GRAY ROAD RETIREMENT COMMUNITY WINDHAM, MAINE

Responsible Party

Owner: Weld, LLC

P.O. Box 1361

Windham, Maine 04062

The owner/applicant is responsible for the maintenance of all stormwater management structures and related site components and the keeping of a maintenance log book with service records until such time that a homeowner's association is created. Records of all inspections and maintenance work performed must be kept on file with the owner and retained for a minimum of five years. The maintenance log will be made available to the upon request. At a minimum, the maintenance of stormwater management systems will be performed on the prescribed schedule.

The procedures outlined in this plan are provided as a general overview of the anticipated practices to be utilized on this site. In some instances, additional measures may be required due to unexpected conditions. The Maine Erosion and Sedimentation Control BMP and Stormwater Management for Maine: Best Management Practices Manuals published by the Maine Department of Environmental Protection (MDEP) should be referenced for additional information.

During Construction

- 1. Inspection and Corrective Action: It is the contractor's responsibility to comply with the inspection and maintenance procedures outlined in this section. Inspection shall occur on all disturbed and impervious areas, erosion control measures, material storage areas that are exposed to precipitation, and locations where vehicles enter or exit the site. These areas shall be inspected at least once a week as well as 24 hours before and after a storm event and prior to completing permanent stabilization measures. A person with knowledge of erosion and stormwater control, including the standards and conditions in the permit, shall conduct the inspections.
- 2. Maintenance: Erosion controls shall be maintained in effective operating condition until areas are permanently stabilized. If best management practices (BMPs) need to be repaired, the repair work should be initiated upon discovery of the problem but no later than the end of the next workday. If BMPs need to be maintained or modified, additional

- BMPs are necessary, or other corrective action is needed, implementation must be completed within seven calendar days and prior to any rainfall event.
- 3. Documentation: A report summarizing the inspections and any corrective action taken must be maintained on site. The log must include the name(s) and qualifications of the person making the inspections; the date(s) of the inspections; and the major observations about the operation and maintenance of erosion and sedimentation controls, materials storage areas, and vehicle access points to the parcel. Major observations must include BMPs that need maintenance, BMPs that failed to operate as designed or proved inadequate for a particular location, and location(s) where additional BMPs are needed. For each BMP requiring maintenance, BMP needing replacement, and location needing additional BMPs, note in the log the corrective action taken and when it was taken. The log must be made accessible to Town staff, and a copy must be provided upon request. The owner shall retain a copy of the log for a period of at least three years from the completion of permanent stabilization.

Housekeeping

- Spill prevention: Controls must be used to prevent pollutants from construction and
 waste materials on site to enter stormwater, which includes storage practices to minimize
 exposure of the materials to stormwater. The site contractor or operator must develop,
 and implement as necessary, appropriate spill prevention, containment, and response
 planning measures.
- 2. Groundwater protection: During construction, liquid petroleum products and other hazardous materials with the potential to contaminate groundwater may not be stored or handled in areas of the site draining to an infiltration area. An "infiltration area" is any area of the site that by design or as a result of soils, topography and other relevant factors accumulates runoff that infiltrates into the soil. Dikes, berms, sumps, and other forms of secondary containment that prevent discharge to groundwater may be used to isolate portions of the site for the purposes of storage and handling of these materials. Any project proposing infiltration of stormwater must provide adequate pre-treatment of stormwater prior to discharge of stormwater to the infiltration area, or provide for treatment within the infiltration area, in order to prevent the accumulation of fines, reduction in infiltration rate, and consequent flooding and destabilization.
- 3. Fugitive sediment and dust: Actions must be taken to ensure that activities do not result in noticeable erosion of soils or fugitive dust emissions during or after construction. Oil may not be used for dust control, but other water additives may be considered as needed. A stabilized construction entrance (SCE) should be included to minimize tracking of mud and sediment. If off-site tracking occurs, public roads should be swept immediately and no less than once a week and prior to significant storm events. Operations during dry months, that experience fugitive dust problems, should wet down unpaved access roads

once a week or more frequently as needed with a water additive to suppress fugitive sediment and dust.

- **4. Debris and other materials:** Minimize the exposure of construction debris, building and landscaping materials, trash, fertilizers, pesticides, herbicides, detergents, sanitary waste and other materials to precipitation and stormwater runoff. These materials must be prevented from becoming a pollutant source.
- 5. Excavation de-watering: Excavation de-watering is the removal of water from trenches, foundations, coffer dams, ponds, and other areas within the construction area that retain water after excavation. In most cases the collected water is heavily silted and hinders correct and safe construction practices. The collected water removed from the ponded area, either through gravity or pumping, must be spread through natural wooded buffers or removed to areas that are specifically designed to collect the maximum amount of sediment possible, like a cofferdam sedimentation basin. Avoid allowing the water to flow over disturbed areas of the site. Equivalent measures may be taken if approved by the Department.
- 6. Authorized Non-stormwater discharges: Identify and prevent contamination by non-stormwater discharges. Where allowed non-stormwater discharges exist, they must be identified and steps should be taken to ensure the implementation of appropriate pollution prevention measures for the non-stormwater component(s) of the discharge. Authorized non-stormwater discharges are:
 - (a) Discharges from firefighting activity;
 - (b) Fire hydrant flushings;
 - (c) Vehicle washwater if detergents are not used and washing is limited to the exterior of vehicles (engine, undercarriage and transmission washing is prohibited);
 - (d) Dust control runoff in accordance with permit conditions and Appendix (C)(3);
 - (e) Routine external building washdown, not including surface paint removal, that does not involve detergents;
 - (f) Pavement washwater (where spills/leaks of toxic or hazardous materials have not occurred, unless all spilled material had been removed) if detergents are not used;
 - (g) Uncontaminated air conditioning or compressor condensate;
 - (h) Uncontaminated groundwater or spring water;
 - (i) Foundation or footer drain-water where flows are not contaminated;
 - (j) Uncontaminated excavation dewatering (see requirements in Appendix C(5));
 - (k) Potable water sources including waterline flushings; and
 - (I) Landscape irrigation.
- 7. Unauthorized non-stormwater discharges: Approval from the Town does not authorize a discharge that is mixed with a source of non-stormwater, other than those discharges in compliance with Section 6 above. Specifically, the Town's approval does not authorize discharges of the following:

- (a) Wastewater from the washout or cleanout of concrete, stucco, paint, form release oils, curing compounds or other construction materials;
- (b) Fuels, oils or other pollutants used in vehicle and equipment operation and maintenance;
- (c) Soaps, solvents, or detergents used in vehicle and equipment washing; and
- (d) Toxic or hazardous substances from a spill or other release.

Post construction

- 1. Inspection and Corrective Action: All measures must be maintained by the owner in effective operating condition. A person with knowledge of erosion and stormwater control, including the standards and conditions of the permit, shall conduct the inspections. The following areas, facilities, and measures must be inspected, and identified deficiencies must be corrected. Areas, facilities, and measures other than those listed below may also require inspection on a specific site.
 - **A. Vegetated Areas:** Inspect vegetated areas, particularly slopes and embankments, early in the growing season or after heavy rains to identify active or potential erosion problems. Replant bare areas or areas with sparse growth. Where rill is evident, armor the area with an appropriate lining or divert the erosive flows to on-site areas able to withstand the concentrated flows.
 - **B.** Ditches, Swales, and Open Channels: Inspect ditches, swales, and other open channels in the spring, late fall, and after heavy rains to remove any obstructions to flow, remove accumulated sediments and debris, control vegetative growth that could obstruct flow, and repair any erosion of the ditch lining. Vegetated ditches must be mowed at least annually or otherwise maintained to control the growth of woody vegetation and maintain flow capacity. Any woody vegetation growing through riprap linings must also be removed. Repair any slumping side slopes as soon as practicable. If the ditch has a riprap lining, replace riprap on areas where any underlying filter fabric or underdrain gravel is showing through the stone or where stones have dislodged. The channel must receive adequate routine maintenance to maintain capacity and prevent or correct any erosion of the channel's bottom or side slopes.
 - **C. Culverts:** Inspect culverts in the spring, late fall, and after heavy rains to remove any obstructions to flow; remove accumulated sediments and debris at the inlet, at the outlet, and within the conduit; and to repair any erosion damage at the culvert's inlet and outlet.
 - **D. Catch Basins:** Inspect and, if required, clean out catch basins at least once a year, preferably in early spring. Clean out must include the removal and legal disposal of any accumulated sediments and debris at the bottom of the basin, at any inlet grates, at any inflow channels to the basin, and at any pipes between basins. If the basin

- outlet is designed to trap floatable materials, then remove the floating debris and any floating oils (using oil-absorptive pads).
- E. Underdrained Filter Basins and Bio-Retention Cell: Basins and Bio-Retention Cell should be inspected semi-annually and following major storm events for the first year and every six months thereafter. The basins and bio-retention cell should drain within 48 hours following a one-inch storm and if a larger storm fills the system to overflow, it shall drain within 36 to 60 hours. If ponding exceeds 48 hours, the top of the filter bed must be rototilled to reestablish the soil's filtration capacity. If water ponds on the surface of the bed for more than 72 hours, the top several inches of the filter shall be replaced with fresh material. Inspect for debris and sediment build up in the forebay, basins and bio-retention cell; and remove as needed. Mowing of the basins and bio-retention cell can only occur semi-annually to a height of no less than 6 inches utilizing a hand-held string trimmer or push-mower. Any bare areas or erosion rills shall be repaired with new filter media or sandy loam then seeded and mulched. The basins and bio-retention cell should also be inspected annually for destabilization of side slopes, embankment settling and other signs of structural failure.
- **F. Roofline Dripedge:** The dripedges should be inspected semi-annually and following major storm events for the first year and every six months thereafter. The reservoir crushed stone should drain within 48 hours following a one-inch storm and if a larger storm fills the system to overflow, it shall drain within 36 to 60 hours. If ponding exceeds 48 hours, the stone reservoir course shall be removed and the filter bed be rototilled to reestablish the soil's filtration capacity. If water ponds in the reservoir course for more than 72 hours, the top several inches of the filter shall be replaced with fresh material. Inspect for debris and sediment build up at surface and remove as needed. The dripedges are part of the stormwater management plan and cannot be paved over or altered in anyway.
- **G. Regular Maintenance:** Clear accumulations of winter sand along roadway once a year, preferably in the spring. Accumulations on pavement may be removed by pavement sweeping. Accumulations of sand along pavement shoulders may be removed by grading excess sand to the pavement edge and removing it manually or by a front-end loader.
- H. Documentation: Keep a log (report) summarizing inspections, maintenance, and any corrective actions taken. The log must include the date on which each inspection or maintenance task was performed, a description of the inspection findings or maintenance completed, and the name of the inspector or maintenance personnel performing the task. If a maintenance task requires the clean-out of any sediments or debris, indicate where the sediment and debris was disposed after removal. The log must be made accessible to Town staff upon request. The permittee shall retain a copy of the log for a period of at least five years from the completion of permanent stabilization. Attached is a sample log.

MAINTENANCE LOG

GRAY ROAD RETIREMENT COMMUNITY WINDHAM, MAINE

The following stormwater management and erosion control items shall be inspected and maintained as prescribed in the Maintenance Plan with recommended frequencies as identified below. The owner is responsible for keeping this maintenance log on file for a minimum of five years and shall provide a copy to the Town upon request. Inspections are to be performed by a qualified third party inspector and all corrective actions shall be performed by personnel familiar with stormwater management systems and erosion controls.

Maintenance	Maintenance Event	Date	Responsible	Comments
Item		Performed	Personnel	
Vegetated	Inspect slopes and			
Areas	embankments early in			
	Spring.			
Ditches,	Inspect after major			
swales, and	rainfall event producing			
other open	1" of rain in two hours.			
channels	Inspect for erosion or			
channels	slumping & repair			
	Mowed at least			
	annually.			
Culverts	Inspect semiannually			
	and after major rainfall.			
	Repair erosion at inlet			
	or outlet of pipe.			
	Repair displaced riprap.			
	Clean accumulated			
	sediment in culverts			
	when >20% full.			
Catch Basins	Inspect to ensure that			
	structure is properly			
	draining.			
	Remove accumulated			
	sediment semiannually.			
	Inspect grates/inlets			
	and remove debris as			
	needed.			

MAINTENANCE LOG

GRAY ROAD RETIREMENT COMMUNITY WINDHAM, MAINE

Maintenance	Maintenance Event	Date	Responsible	Comments
Item		Performed	Personnel	
Underdrained Filter Basin, Bio-Retention Cell And Roofline Dripedges	Check after each rainfall event to ensure that pond drains within 24-48 hours. Replace top several inches of filter if pond does not drain within 72 hours. Mow grass no more than twice a year to no less than 6 inches in height. Inspect semi-annually for erosion or sediment accumulation and repair			
Regular Maintenance	as necessary. Clear accumulation of winter sand in paved areas annually.			